



FOURTH QUARTER MONITORING REPORT
OCTOBER TO DECEMBER 2002
KIN-BUC LANDFILL OPERABLE UNITS 1 AND 2

Prepared for
SCA Services, Inc.
Edison Township, Middlesex County, New Jersey
January 2003

Prepared by
EMCON/OWT, Inc.
Crossroads Corporate Center
One International Boulevard, Suite 700
Mahwah, New Jersey 07495

OWT Project 791186

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CONTENTS

LIST OF TABLES AND ILLUSTRATIONS	iv
SUMMARY	v
1 INTRODUCTION	1-1
2 DESCRIPTION OF MONITORING PROGRAM	2-1
2.1 Hydrogeologic background	2-1
2.2 Remedial Objectives	2-1
2.3 Hydraulic Control and Monitoring System	2-2
2.4 Fourth Quarter Hydraulic Monitoring Activities	2-3
2.5 Continuous Hydraulic Monitoring Results vs. Manual Elevation Measurements	2-4
3 HYDRAULIC MONITORING	3-1
3.1 Assessment of Hydraulic Conditions in the Refuse Unit	3-1
3.2 Assessment of Hydraulic Conditions in the Sand & Gravel Unit	3-2
3.3 OU2 Hydraulic Monitoring	3-5
4 LEACHATE WITHDRAWAL/GROUNDWATER PUMPING	4-1
5 LANDFILL GAS MIGRATION MONITORING	5-1
5.1 Landfill Gas Migration	5-1
5.2 Gas Monitoring Well Results	5-1
5.3 Operational Flare Monitoring Results	5-2
6 CONCLUSIONS	6-1
REFERENCES	
FIGURES	
DRAWING	
TABLES	

CONTENTS (Continued)

APPENDIX A - OU1 REFUSE WELLS CONTINUOUS WATER LEVEL MONITORING RESULTS

APPENDIX B - MONTHLY HYDRAULIC EVALUATIONS

TABLES AND ILLUSTRATIONS

Tables

- 2-1 OU1 Hydraulic Monitoring Well Network/Transects
- 2-2 OU2 Hydraulic Monitoring Network
- 2-3 Fourth Quarter 2002 Manually Recorded Water Levels
- 2-4 Minimum/Maximum Monthly Water Elevations
- 2-5 Troll Water Level Elevations vs. Manual Water Elevations
- 2-6 Leachate Cleanout Monitoring
- 5-1 Landfill Gas Migration Monitoring Well Network/Results

Figure

- 1-1 OU2 Groundwater Monitoring Locations

Drawing

- 1-1 OU1 Site Map

In map pocket

EXECUTIVE SUMMARY

The Kin-Buc Landfill Site is a closed 200-acre industrial/commercial landfill located in Edison, New Jersey, which the USEPA placed on the National Priorities List (NPL) in 1981. A Remedial Investigation/Feasibility Study (RI/FS) was conducted between 1983 and 1988 which resulted in a Record of Decision (ROD) by USEPA in 1990 that called for source control of Operable Unit 1 (OU1).

The remedial action specified in the ROD for OU1 included the construction of a slurry wall around OU1, the collection and treatment of leachate and groundwater from within the containment area, and the capping of the area within the slurry wall. Remedial construction activities for OU1 were completed by the end of August 1995.

In accordance with the RODs, hydraulic monitoring and landfill gas monitoring is conducted on a quarterly basis to evaluate the effectiveness of the remedial actions. This report documents the results of the monitoring activities for the Fourth Quarter of 2002. This report does not include the annual groundwater monitoring data as the annual groundwater sampling and analysis was performed by others.

Remedial Objectives

The general remedial objectives of the OU1 closure and collection systems are to contain source leachate and contaminated groundwater, and to prevent further migration of site-related contaminants. The primary objective of the leachate collection system is to impose an inward gradient as measured across the slurry wall in the refuse unit. The primary objectives of the groundwater collection system are to prevent migration of contaminated groundwater towards the slurry wall and impose an upward gradient from the bedrock unit to the sand & gravel unit.

Hydraulic Control and Monitoring System

The hydraulic control system for OU1 consists of leachate and groundwater collection systems. The leachate collection system consists of a perforated pipe that runs parallel to the inside of the perimeter slurry wall and 4 pump stations. The groundwater collection system consists of 4 pumping wells.

The hydraulic monitoring system for OU1 is located along the circumferential slurry wall with many of the wells located in 5 clusters, called transects. The hydraulic monitoring

wells at the transects are installed in pairs, within the same hydrogeologic unit, with 1 well inside and 1 well outside the circumferential slurry wall. Twenty-four of the monitoring wells are continuously monitored using water level recorders.

The hydraulic monitoring network consists of wells screened in the refuse, sand & gravel, and bedrock units. Well designations of G, S or R; denote hydraulic units of refuse, sand & gravel or bedrock, respectively.

The OU2 hydraulic monitoring well network is located in the Low-Lying Area and Mound B, and monitors groundwater elevations outside of the OU1 containment area.

Fourth Quarter Hydraulic Monitoring Activities

Hydraulic monitoring was performed during the period from October through December 2002.

Hydraulic monitoring indicates that intragradient conditions in the refuse unit (lower water levels in the refuse inside the wall relative to water levels outside the wall) were maintained at TL Nos. 2, 3, 4 and 5, throughout the quarter. Although there are fluctuations in the hydrographs, where groundwater sampling took place and where trolls were removed for inspection, intragradient conditions were maintained overall at these transect locations for the quarter. The fact that the leachate collection system is functioning properly suggests that intragradient conditions are being maintained in the refuse unit at TL No. 1, even though review of the hydrographs does not consistently indicate this condition. Water level elevation measurements taken from Leachate Collection Cleanout Nos. 14 through 16 are included in Table 2-6, and indicate that the leachate collection system is functioning properly.

Hydraulic control was maintained within OU-1 based on the analysis of the significant influence of S&G #2 in acting as a hydraulic sink for sand and gravel and bedrock groundwater. Groundwater flow in the sand and gravel and bedrock is ultimately captured by the pumping well resulting in overall containment of groundwater in OU-1.

Leachate Withdrawal/Groundwater Pumping

The fourth quarter average daily groundwater extraction rate for all of the wells was 23,889 gpd. The total volume of groundwater collected for the quarter was 2,197,754 gallons. Leachate was collected at an average daily rate of 1,662 gpd for the quarter, and the total volume of leachate collected was 152,928 gallons.

Landfill Gas Monitoring

Combustible gas was not detected in any of the 6 gas monitoring wells located on the north side of OU1. Based on the non-detection of combustible gas in the monitoring

wells, the active gas collection system is functioning properly and there is no apparent off-site gas migration. Monitoring at the flare inlet port by landfill personnel throughout the quarter indicated that the landfill gas collection system was delivering an average of 52.8 percent combustible gas to the flare.

1 INTRODUCTION

The Kin-Buc Landfill Site is a closed 200-acre industrial/commercial landfill located in Edison, New Jersey, which operated under a New Jersey Department of Environmental Protection (NJDEP) permit until 1976. The USEPA placed the Kin-Buc Landfill on the National Priorities List (NPL) in 1981. Between 1983 and 1988, the Respondents conducted a Remedial Investigation/Feasibility Study (RI/FS) which resulted in a Record of Decision (ROD) by USEPA in 1990 which called for source control of Operable Unit 1 (OU1), and an additional RI/FS to determine the nature and extent of contamination outside the source area, thus defining Operable Unit 2 (OU2).

Operable Unit 1 includes both Kin-Buc I and II Mounds, the former Pool C Area and a portion of the Low-Lying Area between Kin-Buc I and the Edison Landfill. The remedial action specified in the ROD for OU1 included the construction of a slurry wall around OU1, the collection and treatment of leachate and groundwater from within the containment area, and the capping of the area within the slurry wall.

Operable Unit 2 includes Mound B, Edmonds Creek and adjacent wetlands, the remaining Low-Lying Area between OU1 and the Edison Landfill, Martins Creek, and the Raritan River. The OU2 ROD called for the excavation and disposal of PCB-contaminated sediments from within the Edmonds Creek Marsh Area, the restoration of disturbed wetland areas, and groundwater/surface water monitoring.

Remedial construction activities for both OU1 and OU2 were completed by the end of August 1995.

In accordance with the RODs, hydraulic monitoring and landfill gas monitoring is conducted quarterly to evaluate the effectiveness of the remedial actions. This report documents the results of the monitoring activities for the Fourth Quarter of 2002.

2 DESCRIPTION OF MONITORING PROGRAM

2.1 Hydrogeologic background

The primary hydrogeologic units within OU1 from ground surface downward are refuse, meadow mat, sand & gravel, and bedrock. Near the northern portion of the site the bedrock is closer to the surface and there is no sand & gravel unit in that area.

The southern portion of the site is located in close proximity to the Raritan River. As a result, monitoring wells located on the southern side of OU1 are impacted by tidal fluctuations.

2.2 Remedial Objectives

The general remedial objectives of the OU1 closure and collection systems are to contain source leachate and contaminated groundwater, and to prevent further migration of site-related contaminants. The specific remedial objectives for the leachate collection, groundwater collection, and hydraulic monitoring are summarized as follows:

Aqueous Leachate Collection

- Primary
 - Collect leachate from the refuse unit within the perimeter slurry wall to impose an inward gradient as measured across the slurry wall (hydraulic containment).
- Additional Benefit
 - Reduce the downward gradient between the refuse unit and the underlying sand & gravel or bedrock units.

Sand & Gravel Groundwater Collection (in Primary OU1 Containment)

- **Primary**
 - Prevent migration of contaminated groundwater towards the slurry wall.
 - Impose an upward gradient from the bedrock unit to the sand & gravel unit (hydraulic containment).
- **Additional Benefit**
 - Impose an inward gradient within the sand & gravel unit as measured across the perimeter slurry wall (hydraulic containment).

Sand & Gravel Aquifer Groundwater Collection (in Oil Seeps Area Containment)

- Collect sand & gravel groundwater from within the Oil Seeps Area if an upward gradient between the sand & gravel and the refuse units cannot be imposed by leachate collection alone.

2.3 Hydraulic Control and Monitoring System

The hydraulic control system for OU1 consists of 4 leachate pump stations and 4 sand & gravel groundwater pumping wells. The leachate collection system consists of a perforated pipe that runs parallel to the inside of the perimeter slurry wall. In addition, a corrugated oily leachate collection conduit is located along the south side of Kin-Buc I mound. The layout of the collection system is shown on Drawing 1.

The hydraulic monitoring system for Operable Unit 1 is located along the circumferential slurry wall with many of the wells located in 5 clusters, called transects. The OU1 hydraulic monitoring well network consists of 11 wells screened in the refuse/fill, 8 wells screened in the sand & gravel, and 10 wells screened within bedrock. A summary of the well network is provided in Table 2-1, and the well locations are shown on Drawing 1.

The hydraulic monitoring wells at the transects are installed in pairs, within the same hydrogeologic unit, with 1 well inside and 1 well outside the circumferential slurry wall. The design of the well network allows groundwater elevations to be monitored on either side of the slurry wall and provides data to evaluate the performance of the slurry wall as a hydraulic barrier.

At TL Nos. 2, 3 and 4, the hydraulic monitoring wells are installed in the refuse, sand & gravel, and bedrock units. At TL Nos. 1 and 5, the hydraulic monitoring wells are installed only in the refuse and bedrock units due to the absence of sand and gravel.

deposits in these areas. Well designations of G, S and R, denote hydraulic units of refuse, sand & gravel, and bedrock, respectively.

The OU2 hydraulic monitoring well network is located in the Low-Lying Area and Mound B, and monitors groundwater elevations outside of the OU1 containment area. The hydraulic monitoring system for OU2 consists of 16 wells, as indicated in Table 2-2 and as shown on Figure 1-1. Water elevation measurements from the OU2 wells are taken manually, concurrent with the OU1 monitoring activities.

2.4 Fourth Quarter Hydraulic Monitoring Activities

Hydraulic monitoring for the Fourth Quarter of 2002 (October to December) took place according to the procedures and methods outlined in the Draft Operations and Maintenance (O&M) Manual for the Kin-Buc Landfill, prepared on behalf of the Respondents by Wheelabrator EOS in September 1995 and modified by a letter to EPA dated February 28, 1996.

Components of the hydraulic monitoring program consist of continuous and manual water level measurements. Manual measurements were obtained with an electronic water level indicator. Continuous water levels were obtained at 1-hour intervals using 24 In-Situ "miniTROLL", Model SSP-100 data loggers and transducers.

Several maintenance activities were performed on the miniTROLLS. The miniTROLL, which had previously malfunctioned in Well 15S (serial number 7573) was replaced by In-Situ, Inc. with a new unit (serial number 10275). The replacement miniTroll was installed by EMCON/OWT, Inc. during the December 6, 2002 site visit. Also, In-Situ, Inc. repaired the miniTROLL that had malfunctioned in Well 13G (serial number 6171) and this unit will be installed during the next site visit in early February. Based on the memory failure of the miniTROLL, no continuous electronic data was collected for the months of October and November. However, manual data was taken during each site visit. An SP 4000 Troll was temporarily installed in Well 13G during the December 6, 2002 site visit to collect data until the dedicated miniTroll is repaired.

Three months of continuous water level data have been obtained from the refuse and sand & gravel wells at the site from October 1, 2002 to December 31, 2002. The minimum, maximum, and average recorded water elevations for each month in the quarter are provided in Table 2-4. Continuous groundwater elevation graphs organized by transect location and hydrogeologic unit are provided in Appendix A. Evaluations of the recorded data are performed on a monthly basis and sent to Waste Management. Copies of these monthly evaluations are provided in Appendix B.

Manual groundwater elevation measurements were obtained from the monitoring wells in OU1 and OU2 during site visits on September 26, 2002, November 6, 2002, and December 6, 2002. The manually recorded water level monitoring results are provided in Table 2-3.

2.5 Continuous Hydraulic Monitoring Results vs. Manual Elevation Measurements

The continuous water level monitoring information collected by the Trolls was compared with the data collected from the manual recordings to provide information on the relative accuracy of manual versus automatic recordings. Table 2-5 shows the difference between the manual water level elevation measurements and Troll recordings for the same day and hour. Differences between the manual and continuous measurements were below 0.2 feet for all wells. Based on the comparison above, the data recorded by the Trolls is satisfactory and reflects accurate groundwater elevations.

3 HYDRAULIC MONITORING

The following presents an evaluation of the results of hydraulic monitoring performed during the fourth quarter 2002.

3.1 Assessment of Hydraulic Conditions in the Refuse Unit

As defined in the Record of Decision (ROD) for OU-1, the performance objective for the refuse unit calls for the pumping of leachate to establish inward gradients across the slurry wall with the additional benefit of reducing downward flow into the underlying sand and gravel unit. Based on the hydrographs the following is presented.

TL No. 1 (Well 1G/Well 2G) – Hydrograph No. 1

Intragradiant conditions were not consistently observed throughout the quarter. The average quarterly water elevations for Wells 1G (inside) and 2G (outside) were 11.68 and 12.00 feet msl, respectively. The average head elevation difference between the two wells was approximately 0.32 feet in an inward direction. High water levels in Well 1G have been observed on several previous occasions and may be related to localized conditions around the well.

Water level elevation measurements taken from Leachate Collection Cleanout Nos. 14 through 16 are included in Table 2-6, and indicate that the leachate collection system is functioning properly. The water level elevations observed for Leachate Collection Cleanouts 14 through 15 are all between 9.48 and 10.23 feet msl, and the water level elevations for Cleanouts 16N and 16E were dry (less than the cleanouts invert elevation). This indicates that groundwater flow at this location is from the inside to the Leachate Collection Cleanouts. The leachate collection system is therefore functioning properly and suggests significant capture of leachate. Appendix B (Monthly Hydraulic Evaluations) provides an analysis of the hydraulic performance at Transect 1.

TL No. 2 (Well 3G/Well 4G) – Hydrograph No. 2

Intragradiant conditions were maintained at TL No. 2 in the refuse unit throughout the quarter. The average quarterly water elevations for Wells 3G (inside) and 4G (outside) were 9.93 and 11.17 feet msl, respectively. The average head elevation difference between the two wells was approximately 1.24 feet in an inward direction.

TL No. 3 (Well 5G/Well 6G) – Hydrograph No. 3

Intragradiant conditions were maintained at TL No. 3 in the refuse unit throughout the quarter. The average quarterly water elevations for Wells 5G (inside) and 6G (outside) were 4.35 and 13.35 feet msl, respectively. The head elevation difference between the two wells was approximately 9 feet in an inward direction.

TL No. 4 Well 15G/Well 13G) Oil Seeps Area – Hydrograph No. 4

Intragradiant conditions were maintained at TL No. 4, Oil Seeps Area, in the refuse unit throughout the month of December (data not available for Well 13G for October and November). The average quarterly water elevations for Wells 15G (inside) and 13G (outside) were 1.08 and 5.49 feet msl, respectively. The head elevation difference between the two wells was approximately 4.41 feet in an inward direction. These readings suggest significant intragradiant conditions are being maintained at this location.

TL No. 5 (Well 9G/Well 10G) – Hydrograph No. 5

Intragradiant conditions were maintained at TL No. 5 in the refuse unit throughout the quarter. Although there are fluctuations in the hydrograph where groundwater sampling took place and where the troll was removed for inspection, intragradiant conditions were maintained overall at this location for the quarter. Based on readings from the miniTroll, head levels in well 10G did not equilibrate rapidly following removal, and reinstallation. The average quarterly water elevations for Wells 9G (inside) and 10G (outside) were 7.56 and 7.84 feet msl, respectively. The average head elevation difference between the two wells was approximately 0.28 feet in an inward direction.

3.2 Assessment of Hydraulic Conditions in the Sand & Gravel Unit

For the sand and gravel unit, the performance objectives call for pumping of sand and gravel groundwater to prevent flow toward the slurry wall and to impose upward hydraulic gradients from the bedrock to the sand and gravel. An additional benefit would be the establishment of inward gradients across the slurry wall within the sand and gravel unit. The following is a description of the flow characteristics based on visual observation of the hydrographs.

Horizontal Flow

TL No. 2 (Well 3S/Well 4S) – Hydrograph No. 6

Intragradients were not consistently observed throughout the quarter, although there were periods where intragradients were maintained. The average quarterly water elevations for Wells 3S (inside) and 4S (outside) were 0.61 and 0.82 feet msl, respectively. The average head elevation difference between the two wells was approximately 0.21 feet in an inward direction.

TL No. 3 (Well 5S/Well 6S) – Hydrograph No. 7

Intragradients were maintained at TL No. 3 in the sand & gravel unit throughout the quarter. The average quarterly water elevation for Wells 5S (inside) and 6S (outside) were 1.42 and 5.59 feet msl, respectively. The head elevation difference between the two wells was approximately 4.17 feet in an inward direction.

TL No. 4 (Well 7S/Well 8S) – Hydrograph No. 8

Intragradients were maintained at TL No. 4 in the sand & gravel unit throughout the quarter. The average quarterly water elevation for Wells 7S (inside) and 8S (outside) was 1.70 and 2.51 feet msl, respectively. The head elevation difference between the two wells was approximately 0.81 feet in an inward direction.

TL No. 4 (Well 15S/Well 13S) Oil Seeps Area – Hydrograph No. 9

Intragradients were not evident throughout the quarter. The average quarterly water elevations for Wells 15S (inside) and 13S (outside) were 5.83 and 2.27 feet msl, respectively. The head elevation difference between the two wells was approximately 3.56 feet in an outward direction. Water levels from Well 15G are included in the hydrograph for comparison.

Vertical Flow

TL No. 2 (Well 3S/Well 3RR) – Inside; (Well 4S/Well 4R) – Outside Hydrograph Nos. 10 and 11

Upward gradient conditions were not consistently observed between the bedrock and overlying sand & gravel units inside the slurry wall at TL No. 2 throughout the quarter. The average quarterly water elevation for Well 3S (sand & gravel) and 3RR (bedrock) was 0.61 and 0.51 feet msl, respectively. The difference in average quarterly water elevations was approximately 0.1 feet in a downward direction.

Upward gradient conditions were observed between the bedrock and overlying sand & gravel units outside the slurry wall. The average quarterly water elevation for Wells 4S (sand & gravel) and 4R (bedrock) was 0.82 and 1.38 feet msl, respectively. The difference in average quarterly water elevations was 0.56 feet in an upward direction.

**TL No. 3 (Well 5S/Well 5R) – Inside; (Well 6S/Well 6R) – Outside
Hydrograph Nos. 12 and 13**

Inside the slurry wall at TL No. 3, slight upward gradient conditions were observed between the bedrock and overlying sand & gravel units throughout the quarter. The average quarterly water elevations for Wells 5S (sand & gravel) and 5R (bedrock) were 1.42 and 1.51 feet msl, respectively. The difference in average quarterly water elevations was less than 0.1 feet.

Outside the slurry wall at TL No. 3, upward gradient conditions were not observed between the bedrock and overlying sand & gravel units. The average quarterly water elevations for wells 6S (sand & gravel) and 6R (bedrock) were 5.59 and 1.75, respectively. The difference in average quarterly water elevations was 3.84 feet.

**TL No. 4 (Well 7S/Well 7R) – Inside; (Well 8S/Well 8RR) – Outside
Hydrograph Nos. 14 and 15**

Slight upward gradient conditions were maintained between the bedrock and overlying sand & gravel units inside the slurry wall at TL No. 4 for the months of October and December. The average quarterly water elevations for Wells 7S (sand & gravel) and 7R (bedrock) were 1.70 and 1.74 feet msl, respectively. The difference in average quarterly water elevations was 0.04 feet.

Outside the slurry wall at TL No. 4, upward gradient conditions were not consistently observed between the bedrock and overlying sand & gravel units, although there appears to be a very slight upward gradient condition. Because the average water elevations are so close, a dominant flow direction cannot be established. The average quarterly water elevations for Wells 8S (sand & gravel) and 8RR (bedrock) were 2.51 feet and 2.48 feet msl, respectively. The difference in average quarterly water elevations was 0.03 feet.

3.2.1 Analysis

While initial review of the hydrographs indicate that certain performance objectives may not be met, (uniform achievement of upward gradients from the bedrock to the sand and gravel, and inward gradients across the slurry wall) containment is still maintained by the pumping wells SG-2 and SG-3. Figures 1 through 4 (See Appendix B) depict horizontal or vertical flow vectors within the sand and gravel or bedrock units. These diagrams show that although downward groundwater flow from the sand and gravel to the bedrock

may occur locally within the slurry wall, the zone of influence of the pumping wells includes the sand and gravel units and the upper portion of the bedrock within the slurry wall. Regardless of whether groundwater is flowing vertically upward or downward within the slurry wall in the sand and gravel and upper bedrock, it will migrate toward the pumping wells, and will be captured. Examination of the pumping results for this quarter indicates that this process is more efficient if SG-3 is pumped in conjunction with SG-2.

3.3 OU2 Hydraulic Monitoring

The synoptic groundwater elevations obtained during the Fourth Quarter of 2002 indicate both upward and downward hydraulic gradients.

4 LEACHATE WITHDRAWAL/GROUNDWATER PUMPING

The performance of the site hydraulic controls is largely dependent upon groundwater pumping and leachate withdrawal rates. The design aqueous leachate and groundwater (GW) collection rates called for a ratio of 3:1, groundwater to leachate of 30,000 gpd groundwater, and 10,000 gpd leachate. The collection rates differed from the design rates due to variations between design assumptions and actual site conditions. Collection rates are also adjusted based on changing site and operational conditions.

Operation records are maintained at the site and contain estimated daily averages for leachate and groundwater withdrawal. The monthly volumes collected and the daily average collection rate are provided below:

Monitoring Period	Groundwater S&G No. 1	Groundwater S&G No. 2	Groundwater S&G No. 3	Groundwater S&G No. 4	Leachate
October	0 gal.	609,084 gal.	199,765 gal.	14,274 gal.	41,831 gal.
	0 gpd	19,648 gpd	6,444 gpd	460 gpd	1,349 gpd
November	0 gal.	485,989 gal.	121,533 gal.	23,784 gal.	47,508 gal.
	0 gpd	16,758 gpd	4,051 gpd	793 gpd	1,583 gpd
December	12,400 gal.	610,296 gal.	120,629 gal.	0 gal.	63,589 gal.
	400 gpd	19,687 gpd	3,891 gpd	0 gpd	2,051 gpd
Quarter	12,400 gal.	1,705,369 gal.	441,927 gal.	38,058 gal.	152,928 gal.
	135 gpd	18,537 gpd	4,803 gpd	414 gpd	1,662 gpd

The volume of groundwater collected in the fourth quarter is 2,197,754 gallons. The average daily groundwater withdrawal rate for the fourth quarter is 23,889 gpd.

5 LANDFILL GAS MIGRATION MONITORING

Landfill gas migration monitoring was performed at the operational flare port inlet and the 6 gas migration monitoring wells located along the northern edge of the landfill boundary.

5.1 Landfill Gas Migration

The purpose of the gas migration monitoring program is to monitor for off-site gas migration in those areas where gas migration or accumulation could lead to explosive conditions. Six gas migration monitoring wells are located outside of the circumferential slurry wall along the northern edge of the landfill boundary. The well locations are depicted on Drawing 1 and are spaced in 200-foot increments.

All areas of OU1 exterior to the slurry wall contain waste materials except along the northern edge of the landfill boundary. High levels of gas are not expected to be detected along the northern boundary because the slurry wall will act as an effective barrier, and the presence of an active gas extraction system and the high water table will inhibit gas migration.

Gas monitoring in other areas of the site containing waste materials will likely reveal combustible gas. However, since no on-site OU1 buildings are present (except the leachate treatment facility, which has its own engineered gas monitoring and control system), gas migration monitoring in the waste areas is not required by the O&M manual.

5.2 Gas Monitoring Well Results

Measurements of percent combustible gas (% GAS) and percent lower explosive limit (% LEL) were performed in the 6 gas migration monitoring wells along the northern boundary of the site on December 6, 2002. The wells were monitored in accordance with Attachment 1, Section 3.0 - Routine Operations and Maintenance of the Kin-Buc Landfill Draft O&M Manual (Wheelabrator, 1995). A Landtec GEM 500 sampling device was used to measure the concentration of combustible gas at each well by attaching the meter's sample tubing to the well head petcock and drawing the sample through the meter. Detectable levels of percent combustible gas and percent lower explosive limit

were not observed in any gas monitoring wells. The results for the 6 gas migration monitoring wells are shown in Table 5-1.

5.3 Operational Flare Monitoring Results

The percent combustible gas by volume (% methane) at the landfill's operational flare port inlet was recorded throughout the fourth quarter of 2002. All readings were collected with a Landtec GEM 500 Gas Analyzer, equipped with a charcoal filter. Monitoring performed on December 6, 2002 revealed combustible gas at 51.4 percent at the flare port inlet.

The following summarizes the flare station operation during the Fourth Quarter of 2002:

Date	Gas Flow (SCFM)	Methane % by volume
10/07/02	125	48.1
10/21/02	124	47.0
11/29/02	119	62.8
11/30/02	135	62.9
12/04/02	117	45.8
12/30/02	118	50.1
Averages for Third Quarter	123	52.8

Note: Flare station data provided by Landfill personnel.

6 CONCLUSIONS

Significant conclusions for the Fourth Quarter of 2002 monitoring program are as follows:

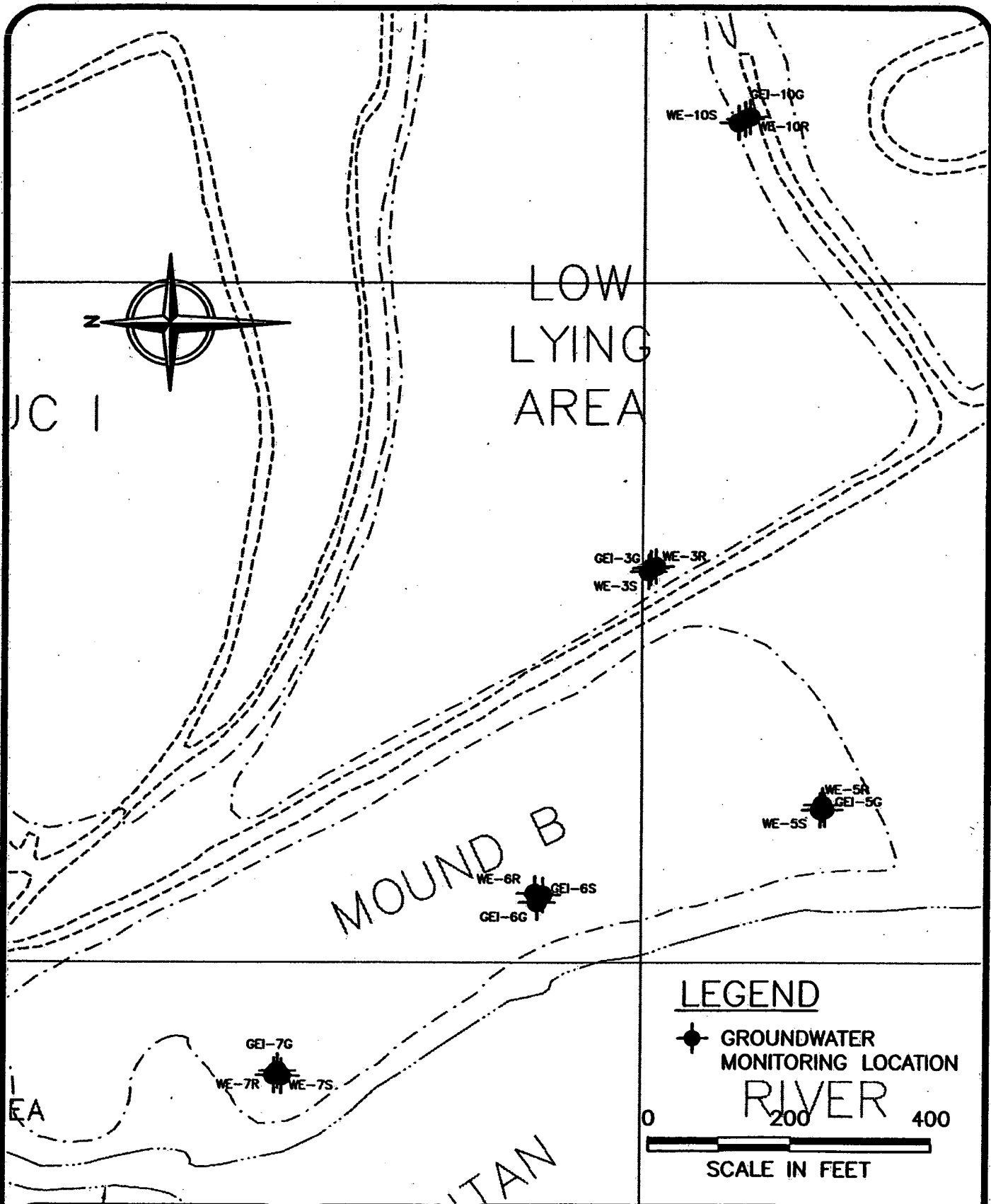
- In the refuse unit, intragradient conditions were maintained over the entire quarter at Transects 2, 3, 4, and 5. An average daily leachate extraction rate of 1,662 gpd was collected.
- Intragradient conditions were not indicated by the monitoring wells in the refuse unit at Transect 1, although levels in the leachate collection system indicate intragradient conditions are present at this location.
- Hydraulic control was maintained within OU-1 based on the analysis of the significant influence of S&G#2 and S&G#3 in acting as a hydraulic sink for sand and gravel and bedrock groundwater. Groundwater flow in the sand and gravel and bedrock is ultimately captured by the pumping well resulting in overall containment of groundwater in OU-1.
- In view of the analysis presented herein, it is recommended that the combined groundwater pumping rates in the sand and gravel be maintained at 15,000 gpd with S&G#2 and S&G#3 pumping at 10,000 gpd and 5,000 gpd, respectively. These lower pumping rates will be evaluated to confirm continued hydraulic control of OU-1 groundwater.
- Maintaining a leachate collection rate of 1,500 gpd is recommended.
- Combustible gas as a percent of total gas and the lower explosive limit was not detected in the 6 monitoring wells located on the northern boundary of the site. The flare was operational and the average percent methane for the quarter at the flare port inlet was 52.8 percent. Based on the non-detection of combustible gas in the monitoring wells, the active gas collection system is functioning properly and there is no off-site gas migration.

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Figure

ENE-MTOWN2/DATA: N:\DWG\12568001\MAKBF-01.dwg Xrefs: MAKBE01, MAKBTW01, MAKBB001
 Scale: 1 = 1.00 DimScale: 1 = 200.00 Date: 11/11/96 Time: 1:36 PM Operator: FDEGEORG



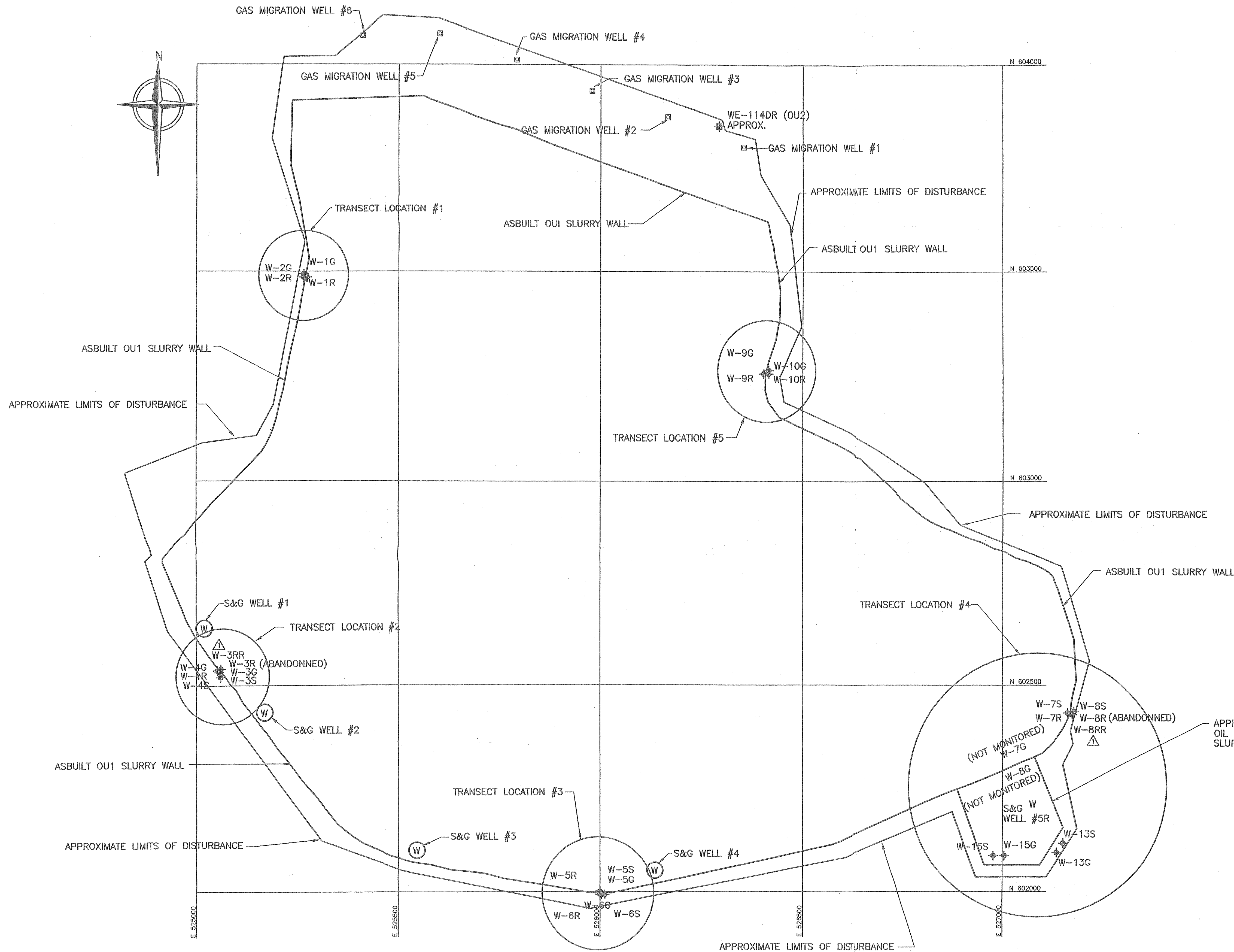
Emcon

DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO.
 12568-001.000

FIGURE 1-1
 KINBUC LANDFILL

EDISON TOWNSHIP, NEW JERSEY
 OU2 GROUNDWATER
 MONITORING LOCATIONS

Drawing



LEGEND

- NEW MONITORING WELLS
- NEW SAND & GRAVEL PUMPING WELLS
- NEW GAS MIGRATION WELLS
- OU2 UPGRADIENT MONITORING BEDROCK WELL (APPROXIMATE LOCATION)

SOURCE: BASEMAP DATA TAKEN FROM PLAN SHEET 10A OF MAP ENTITLED "KINBUC LANDFILL FINAL WELL LOCATION PLAN" PREPARED BY CONTI ENVIRONMENTAL INC. DATED JULY 17, 1995.

- NOTES:
1. THE MONITORING WELL LOCATIONS W-7G, W-8G, W-13G, W-13S, W-15G, AND W-15S ARE APPROXIMATE.
 2. MONITORING WELL W-8R IS DAMAGED AND NOT SERVICABLE AS A WATER QUALITY MONITORING POINT.
 3. MONITORING WELLS W-7G AND W-8G REPLACED BY W-15G AND W-13G.
 4. MONITORING WELLS W-8R AND W-3R ABANDONED AND REPLACED BY NEW MONITORING WELLS W-8RR AND W-3RR RESPECTIVELY.

REV	DATE	DESCRIPTION	DWN BY	DES BY	CHK BY	APP BY
1	2/99	ADD NEW WELLS				
2	5/96	DATE OF ISSUE	SDT	RB	CHK BY	APP BY



KIN-BUC LANDFILL
EXISTING GROUNDWATER MONITORING PLAN
EDISON TOWNSHIP, NEW JERSEY

OPERABLE UNIT 1 MONITORING NETWORK

DRAWING NO. 1
PROJECT NO. 01862-025.000

IMAGE FILE: c:\w\map2
XREF FILE: MACBUC1.dwg
PROJECT: Kin-Buc Landfill - Edison Township, New Jersey
DATE: 5/96
SCALE: 1" = 150'

TABLES

Table 2-1

**Kin-Buc Landfill
Operable Unit 1
Continuous Hydraulic Monitoring Well Network/Transects**

Transect Location No.	Screened Hydrogeologic Unit	Well Location Inside Slurry Wall	Well Location Outside Slurry Wall
1	Refuse/Fill	W-1G	W-2G
2	Refuse/Fill	W-3G	W-4G
	Sand and Gravel	W-3S	W-4S
	Bedrock	W-3RR	W-4R
3	Refuse/Fill	W-5G	W-6G
	Sand and Gravel	W-5S	W-6S
	Bedrock	W-5R	W-6R
4	Refuse/Fill ⁽¹⁾	W-15G	W-13G
	Sand and Gravel ⁽¹⁾	W-15S	W-13S
	Sand and Gravel ⁽²⁾	W-7S	W-8S
	Bedrock ⁽²⁾	W-7R	W-8RR
5	Refuse/Fill	W-9G	W-10G

Notes: ⁽¹⁾ Wells located across the extended slurry wall.

⁽²⁾ Wells located across the OU1 circumferential slurry wall.

Table 2-2

**Kin-Buc Landfill
Operable Unit 2
Hydraulic Monitoring Network**

Well Location	Screened Hydrogeologic Unit
Low-Lying Area	
GEI-10G	Fill/Refuse
WE-10S	Sand & Gravel
WE-10R	Bedrock
GEI-3G	Fill/Refuse
WE-3S	Sand & Gravel
WE-3R	Bedrock
Mound B	
GEI-5G	Fill/Refuse
WE-5S	Sand & Gravel
WE-5R	Bedrock
GEI-6G	Fill/Refuse
GEI-6S	Sand & Gravel
WE-6R	Bedrock
GEI-7G	Fill/Refuse
WE-7S	Sand & Gravel
WE-7R	Bedrock
Upgradient	
WE-114DR	Bedrock

Table 2-3
KinBuc Landfill Operable Units 1 and 2
Modified Monitoring Program
Fourth Quarter 2002
Manually Recorded Water Level Elevations

Well ID	TOC Bottom	TOC Ref Elevation	September 26, 2002		November 6, 2002		December 6, 2002	
			TOC Static	Elevation	TOC Static	Elevation	TOC Static	Elevation
OU1								
W-1G	20.50	30.78	18.54	12.24	18.97	11.81	19.29	11.49
W-1R	35.34	30.79	20.45	10.34	19.68	11.11	20.43	10.36
W-2G	20.38	30.77	20.00	10.77	19.50	11.27	18.39	12.38
W-2R	35.33	30.64	23.45	7.19	22.65	7.99	23.47	7.17
W-3G (oil)	19.07	20.73	10.84	9.89	10.51	10.22	10.95	9.78
W-3G	19.07	20.73	12.65	8.08	11.44	9.29	12.77	7.96
W-3S	31.48	20.79	20.36	0.43	19.48	1.31	20.46	0.33
W-3RR	54.40	21.16	20.65	0.51	19.66	1.50	20.62	0.54
W-4G	17.57	20.23	9.46	10.77	8.81	11.42	9.12	11.11
W-4S	31.58	19.71	18.34	1.37	17.66	2.05	18.04	1.67
W-4R	54.92	20.61	19.31	1.30	18.05	2.56	19.04	1.57
W-5G	24.36	23.94	13.91	10.03	13.54	10.40	14.02	9.92
W-5S	30.33	24.33	23.12	1.21	22.17	2.16	23.16	1.17
W-5R	41.64	24.11	23.05	1.06	22.15	1.96	23.01	1.10
W-6G	23.99	23.69	10.85	12.84	9.87	13.82	10.37	13.32
W-6S	38.49	24.00	22.58	1.42	21.57	2.43	22.55	1.45
W-6R	50.43	23.99	22.52	1.47	21.55	2.44	22.64	1.35
W-7G	19.91	18.30	8.45	9.85	7.92	10.38	8.53	9.77
W-7S	29.34	11.61	10.20	1.41	9.30	2.31	10.18	1.43
W-7R	45.13	11.05	9.53	1.52	8.60	2.45	9.56	1.49
W-8S	28.86	10.92	8.25	2.67	8.29	2.63	8.54	2.38
W-8RR	41.60	9.51	6.83	2.68	6.91	2.60	7.16	2.35
W-9G	21.93	27.34	19.78	7.56	19.45	7.89	19.99	7.35
W-9R	39.05	27.68	21.40	6.28	20.82	6.86	21.27	6.41
W-10G	22.56	27.43	18.84	8.59	18.73	8.70	20.58	6.85
W-10R	34.01	27.43	19.43	8.00	18.91	8.52	19.60	7.83
W-13G	10.30	10.17	3.66	6.51	3.29	6.88	3.29	6.88
W-13S	29.32	10.10	7.75	2.35	7.56	2.54	8.11	1.99
W-15G ⁽¹⁾	16.99	16.18	14.69	1.49	14.57	1.61	14.66	1.52
W-15S	33.36	16.05	13.91	2.14	13.36	2.69	13.90	2.15
OU2								
GEI-10G	13.91	13.65	1.34	12.31	0.58	13.07	1.10	12.55
WE-10S	29.57	14.99	13.41	1.58	12.82	2.17	13.68	1.31
WE-10R	41.74	13.96	12.35	1.61	11.76	2.20	12.61	1.35
GEI-3G	13.54	16.73	4.63	12.10	4.63	12.10	4.11	12.62
WE-3S	25.67	15.12	13.74	1.38	13.62	1.50	14.49	0.63
WE-3R	46.51	14.99	13.37	1.62	13.72	1.27	14.45	0.54
GEI-5G	14.60	16.08	9.31	6.77	9.04	7.04	9.31	6.77
WE-5S	25.84	15.04	12.83	2.21	13.75	1.29	14.39	0.65
WE-5R	49.64	15.31	13.44	1.87	14.23	1.08	14.95	0.36
GEI-6G	14.97	19.76	11.68	8.08	11.58	8.18	11.78	7.98
GEI-6S	43.67	20.99	18.63	2.36	20.75	0.24	21.52	-0.53
WE-6R	47.12	19.62	17.54	2.08	19.72	-0.10	20.45	-0.83
GEI-7G	13.74	17.23	dry	<3.49	dry	<3.49	dry	<3.49
WE-7S	30.07	15.86	12.98	2.88	15.88	-0.02	16.73	-0.87
WE-7R	72.88	15.93	14.04	1.89	14.00	1.93	14.85	1.08
WE-114DR	44.84	23.76	17.84	5.92	17.10	6.66	17.71	6.05

NOTE:

(1) All level, reference, bottom measurements recorded to the top of PVC inner casing.

Table 2-4
KinBuc Landfill Operable Units 1 and 2
Continuous Hydraulic Monitoring Results
Fourth Quarter 2002
Minimum/Maximum/Average Water Elevations

Inside Slurry Wall					Outside Slurry Wall				
Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)	Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)
W-1G	October	11.86	12.18	12.00	W-2G	October	10.26	12.48	11.19
	November	11.57	11.86	11.73		November	10.29	13.36	12.10
	December	11.23	11.61	11.34		December	11.49	13.26	12.59
	4th Quarter	11.23	12.18	11.68		4th Quarter	10.26	13.36	12.00
W-3G	October	9.77	10.27	9.96	W-4G	October	10.78	11.57	11.11
	November	9.67	10.35	9.94		November	3.92	11.80	11.18
	December	9.57	10.31	9.89		December	10.84	11.89	11.23
	4th Quarter	9.57	10.35	9.93		4th Quarter	3.92	11.80	11.17
W-3S	October	0.14	1.83	0.79	W-4S	October	-0.37	2.84	1.09
	November	-9.45	1.91	0.68		November	-11.47	2.63	0.76
	December	-0.64	1.75	0.38		December	-1.15	3.17	0.62
	4th Quarter	-9.45	1.91	0.61		4th Quarter	-11.47	3.17	0.82
W-5G	October	4.18	4.93	4.38	W-6G	October	12.77	14.32	13.34
	November	4.07	4.80	4.39		November	2.84	13.86	13.33
	December	3.96	4.89	4.27		December	12.63	14.23	13.37
	4th Quarter	3.96	4.93	4.35		4th Quarter	2.84	14.32	13.35
W-5S	October	0.94	2.66	1.54	W-6S	October	5.16	6.95	5.78
	November	0.90	2.55	1.47		November	0.00	6.56	5.55
	December	0.28	2.72	1.27		December	4.33	6.96	5.45
	4th Quarter	0.28	2.72	1.42		4th Quarter	0.00	6.96	5.59
W-7S	October	1.35	2.85	1.82	W-8S	October	1.97	4.68	2.69
	November	1.34	2.54	1.76		November	-3.50	4.80	2.44
	December	0.78	2.80	1.53		December	1.66	5.60	2.41
	4th Quarter	0.78	2.85	1.70		4th Quarter	-3.50	5.60	2.51
W-15S	October	5.48	7.11	5.95	W-13S	October	1.88	3.78	2.41
	November	5.45	6.93	5.85		November	-4.33	3.73	2.22
	December	5.02	6.05	5.69		December	1.40	4.25	2.17
	4th Quarter	5.02	7.11	5.83		4th Quarter	-4.33	4.25	2.27
W-15G	October	1.02	1.24	1.11	W-13G	October	NA (1)	NA (1)	6.88 (2)
	November	-0.28	1.32	1.05		November	NA (1)	NA (1)	6.62 (2)
	December	0.86	1.35	1.09		December	2.66	3.51	2.97
	4th Quarter	-0.28	1.35	1.08		4th Quarter	2.66	3.51	5.49
W-9G	October	7.54	7.87	7.71	W-10G	October	8.57	8.72	8.65
	November	7.84	7.87	7.64		November	5.56	8.72	7.22
	December	7.18	7.70	7.34		December	5.73	8.11	7.63
	4th Quarter	7.18	7.87	7.56		4th Quarter	5.56	8.72	7.84
W-3RR	October	-0.37	1.97	0.70	W-4R	October	-0.06	3.37	1.58
	November	-14.21	2.36	0.56		November	-14.83	3.47	1.41
	December	-1.13	2.22	0.29		December	-0.62	3.77	1.17
	4th Quarter	-14.21	2.36	0.51		4th Quarter	-14.83	3.77	1.38

Table 2-4
KinBuc Landfill Operable Units 1 and 2
Continuous Hydraulic Monitoring Results
Fourth Quarter 2002
Minimum/Maximum/Average Water Elevations

Inside Slurry Wall					Outside Slurry Wall				
Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)	Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)
W-5R	October	1.06	2.78	1.67	W-6R	October	1.27	2.93	1.83
	November	-10.59	2.56	1.49		November	-4.23	2.86	1.72
	December	0.28	2.89	1.34		December	0.67	3.21	1.71
	4th Quarter	-10.59	2.89	1.51		4th Quarter	-4.23	3.21	1.75
W-7R	October	1.43	2.92	1.89	W-8RR	October	1.99	4.72	2.72
	November	-7.21	2.52	1.75		November	-18.12	4.82	2.45
	December	0.81	2.88	1.60		December	1.67	5.60	2.48
	4th Quarter	-7.21	2.92	1.74		4th Quarter	1.67	5.60	2.48

Notes:

1. Troll malfunctioned, data was not collected.
2. Water elevation calculated from manual water levels.

Table 2-5
KinBuc Landfill Operable Unit 1
Fourth Quarter 2002
Troll Water Elevations vs. Manual Water Elevations

OU 1 Well ID	November 6, 2002			December 6, 2002			January 2, 2003			Average
	Troll	Manual	Difference	Troll	Manual	Difference	Troll	Manual	Difference	Difference
W-1G	11.87	11.81	0.06	11.50	11.49	0.01	11.28	11.27	0.01	0.03
W-2G	11.27	11.27	0.00	12.37	12.38	0.01	13.30	13.31	0.01	0.01
W-3G	9.34	9.29	0.05	8.01	7.96	0.05	7.97	7.92	0.05	0.05
W-3S	1.28	1.31	0.03	0.32	0.33	0.01	1.20	1.20	0.00	0.01
W-3RR	1.50	1.50	0.00	0.54	0.54	0.00	1.45	1.45	0.00	0.00
W-4G	11.43	11.42	0.01	11.10	11.11	0.01	11.19	11.18	0.01	0.01
W-4S	2.56	2.05	0.51	1.67	1.67	0.00	2.54	2.54	0.00	0.17
W-4R	2.62	2.56	0.06	1.63	1.57	0.06	2.44	2.37	0.07	0.06
W-5G	10.38	10.40	0.02	9.92	9.92	0.00	9.77	9.82	0.05	0.02
W-5S	2.12	2.16	0.04	1.24	1.17	0.07	2.19	2.21	0.02	0.04
W-5R	1.92	1.96	0.04	1.09	1.10	0.01	2.05	2.08	0.03	0.03
W-6G	13.84	13.82	0.02	13.36	13.32	0.04	13.61	13.63	0.02	0.03
W-6S	2.38	2.43	0.05	1.43	1.45	0.02	2.36	2.40	0.04	0.04
W-6R	2.44	2.44	0.00	1.45	1.35	0.10	2.46	2.48	0.02	0.04
W-7S	2.29	2.31	0.02	1.43	1.43	0.00	2.15	2.18	0.03	0.02
W-7R	2.43	2.45	0.02	1.50	1.49	0.01	2.21	2.21	0.00	0.01
W-8S	2.61	2.63	0.02	2.38	2.38	0.00	2.59	2.60	0.01	0.01
W-8RR	2.62	2.60	0.02	2.37	2.35	0.02	2.60	2.54	0.06	0.03
W-9G	7.87	7.89	0.02	7.33	7.35	0.02	7.34	7.36	0.02	0.02
W-10G	8.70	8.70	0.00	6.84	6.85	0.01	8.12	8.04	0.08	0.03
W-13G	NA (1)	6.88	NA (1)	NA (1)	6.62	NA (1)	6.85	6.82	0.03	0.03
W-13S	2.55	2.54	0.01	1.99	1.99	0.00	2.45	2.44	0.01	0.01
W-15G	1.61	1.61	0.00	1.52	1.52	0.00	1.51	1.51	0.00	0.00
W-15S	2.72	2.69	0.03	2.13	2.15	0.02	2.65	2.69	0.04	0.03

Notes : (1) Troll data was not collected due to device malfunction. Water levels taken manually.

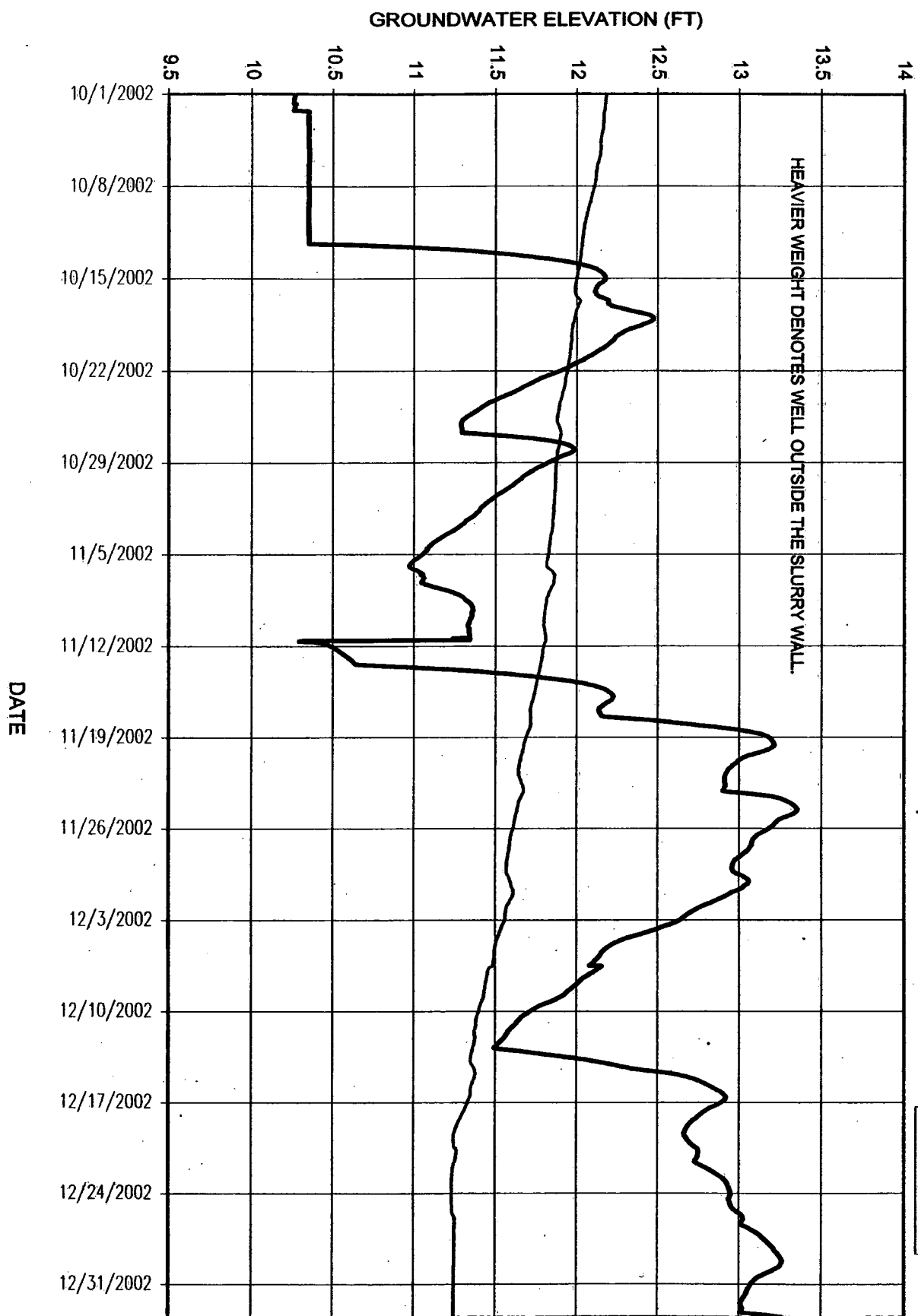
Table 5-1

**Kin-Buc Landfill
Operable Unit 1
Fourth Quarter 2002 Modified Program
Gas Monitoring Well Network/Results**

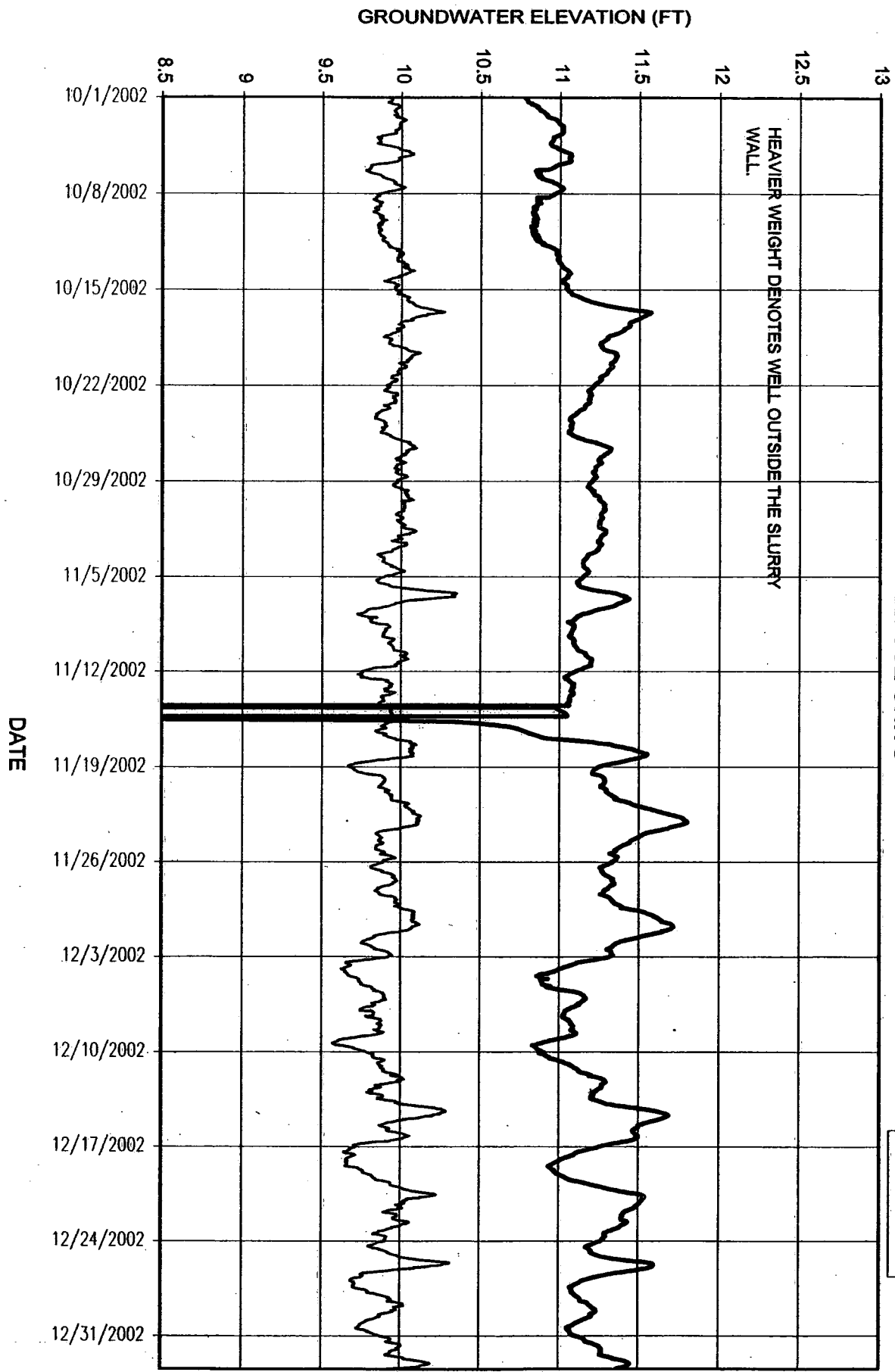
Well (Network) Location	Monitoring Result	
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GMW-02	0	0
GMW-03	0	0
GMW-04	0	0
GMW-05	0	0
GMW-06	0	0
Operational Flare Inlet	NA	51.4

APPENDIX A
CONTINUOUS WATER LEVEL MONITORING RESULTS

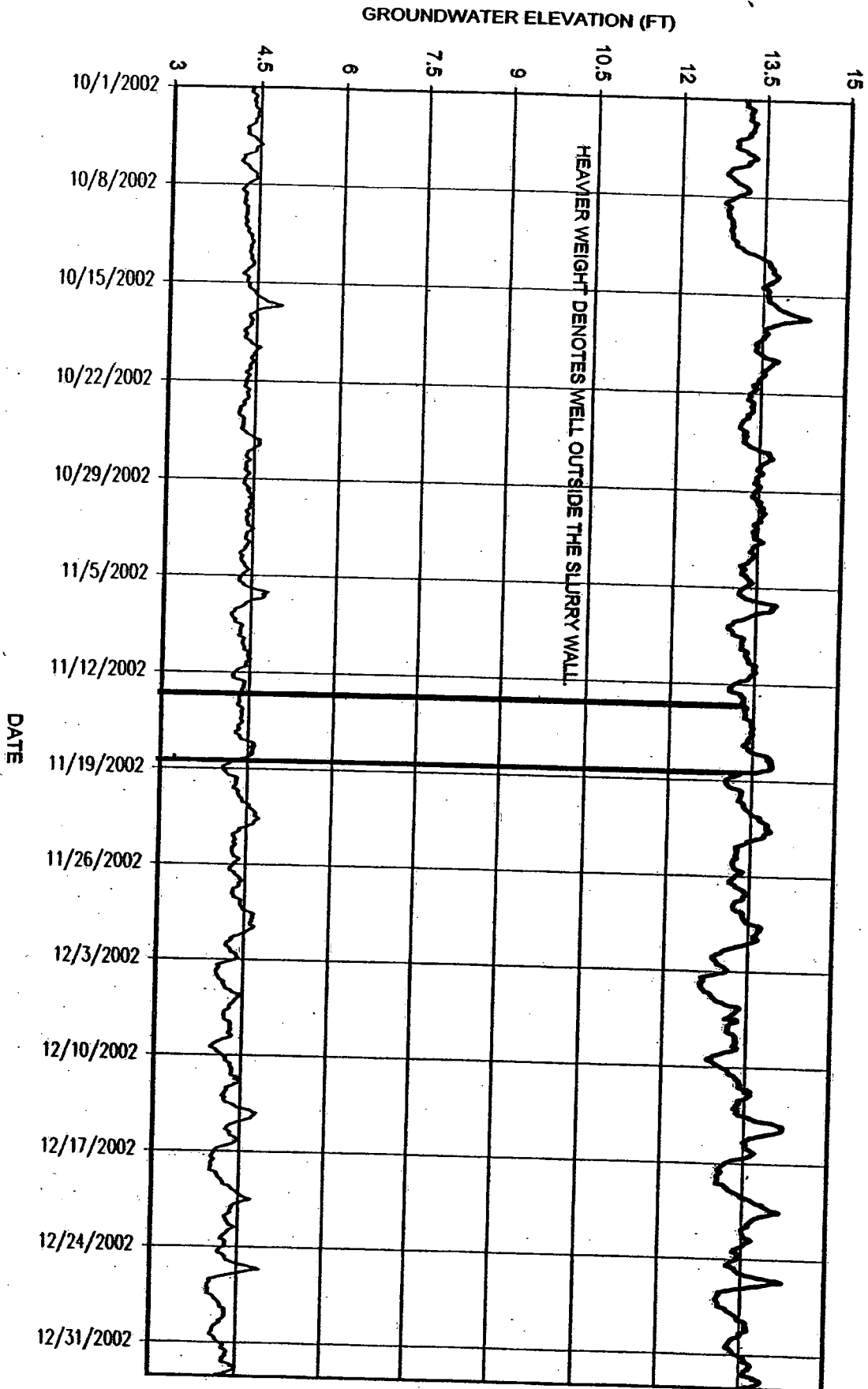
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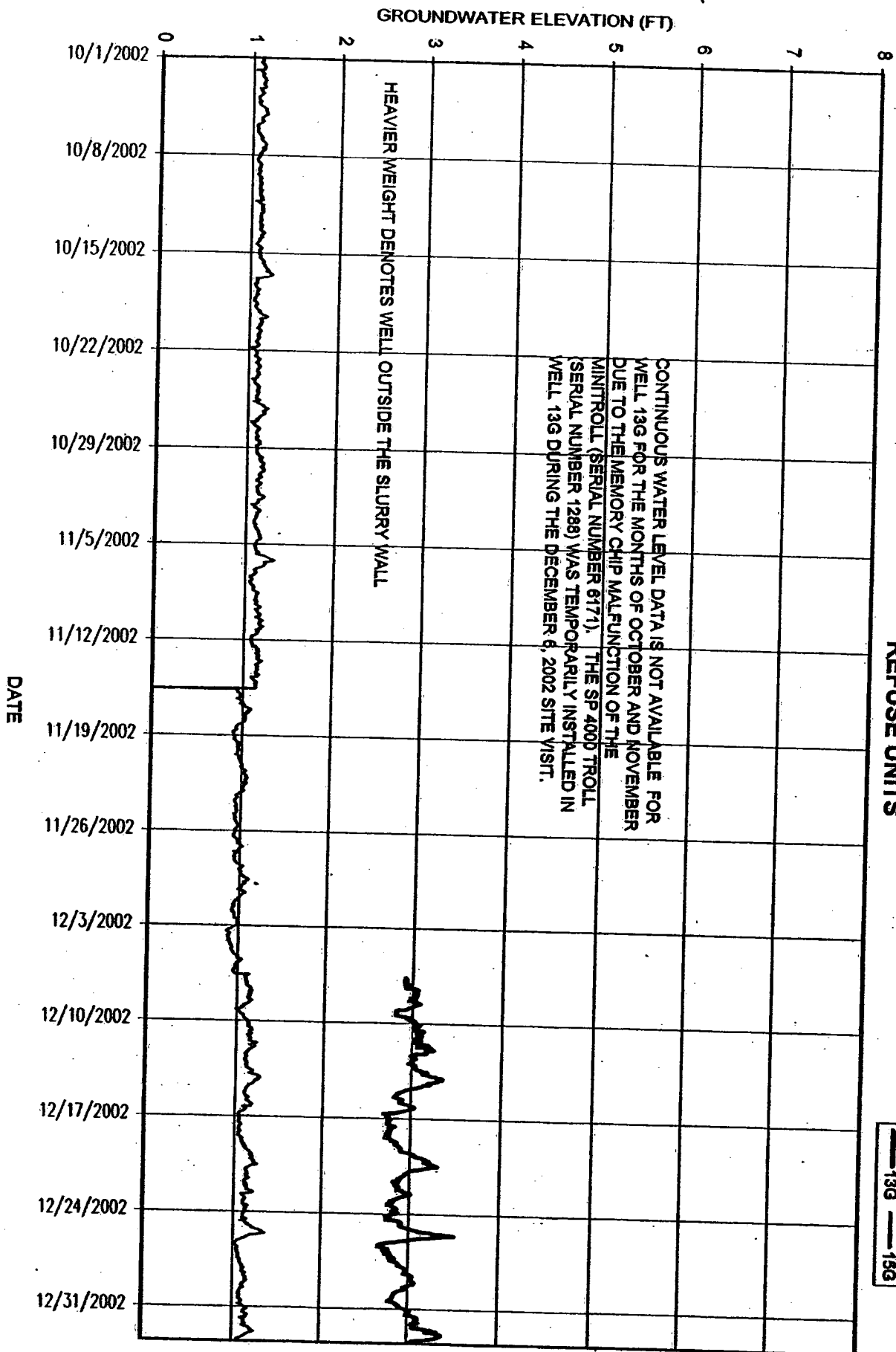
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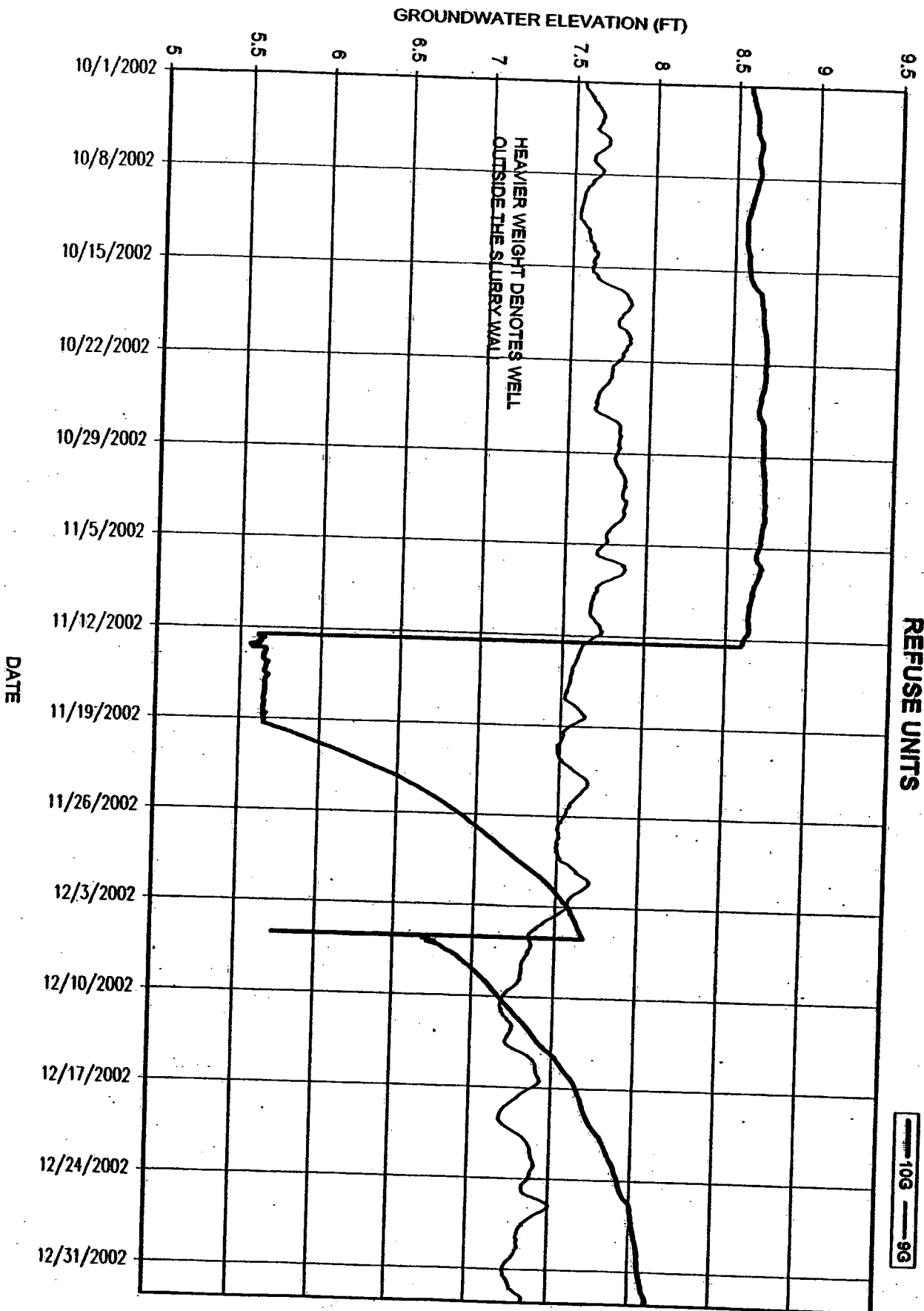
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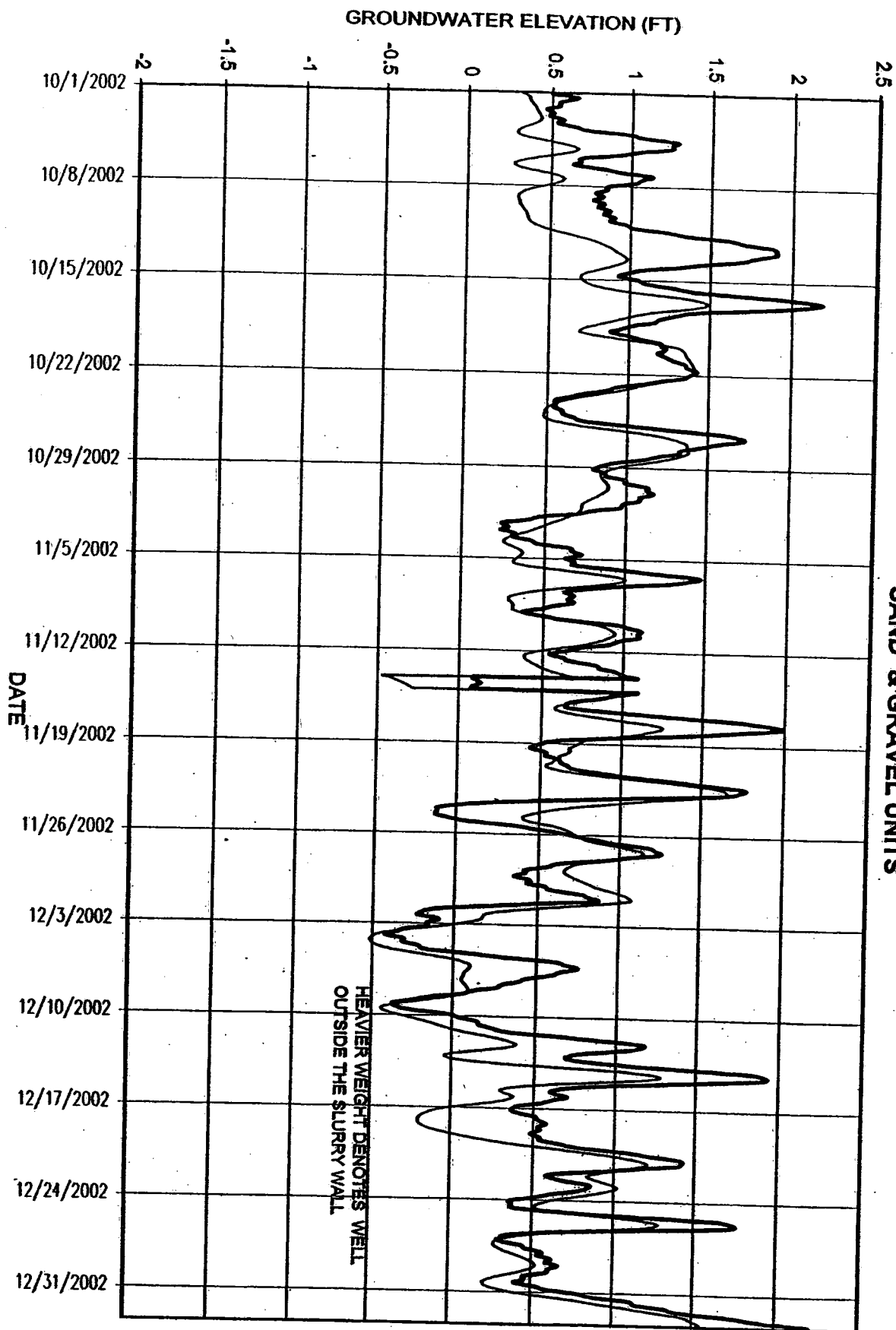
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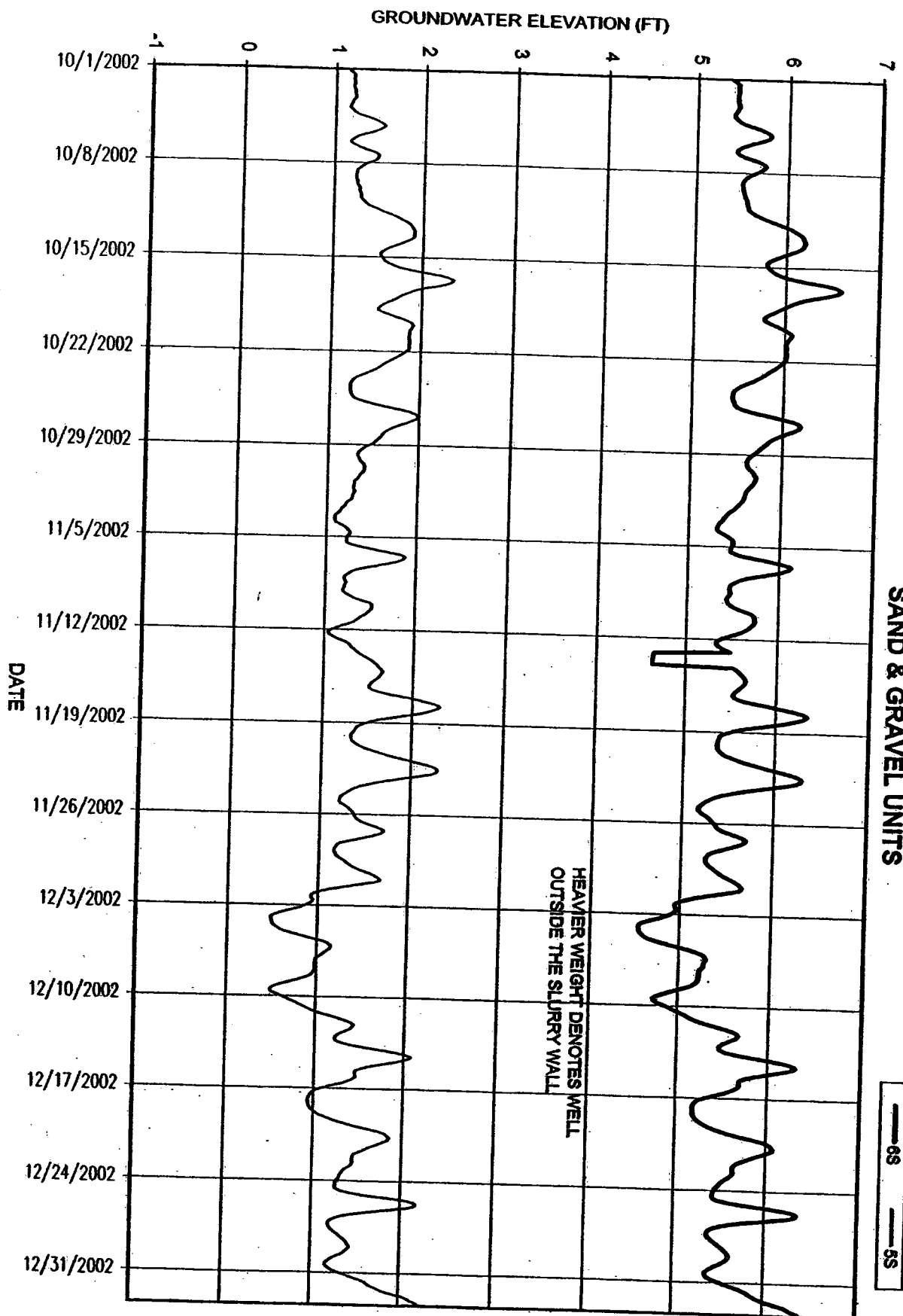
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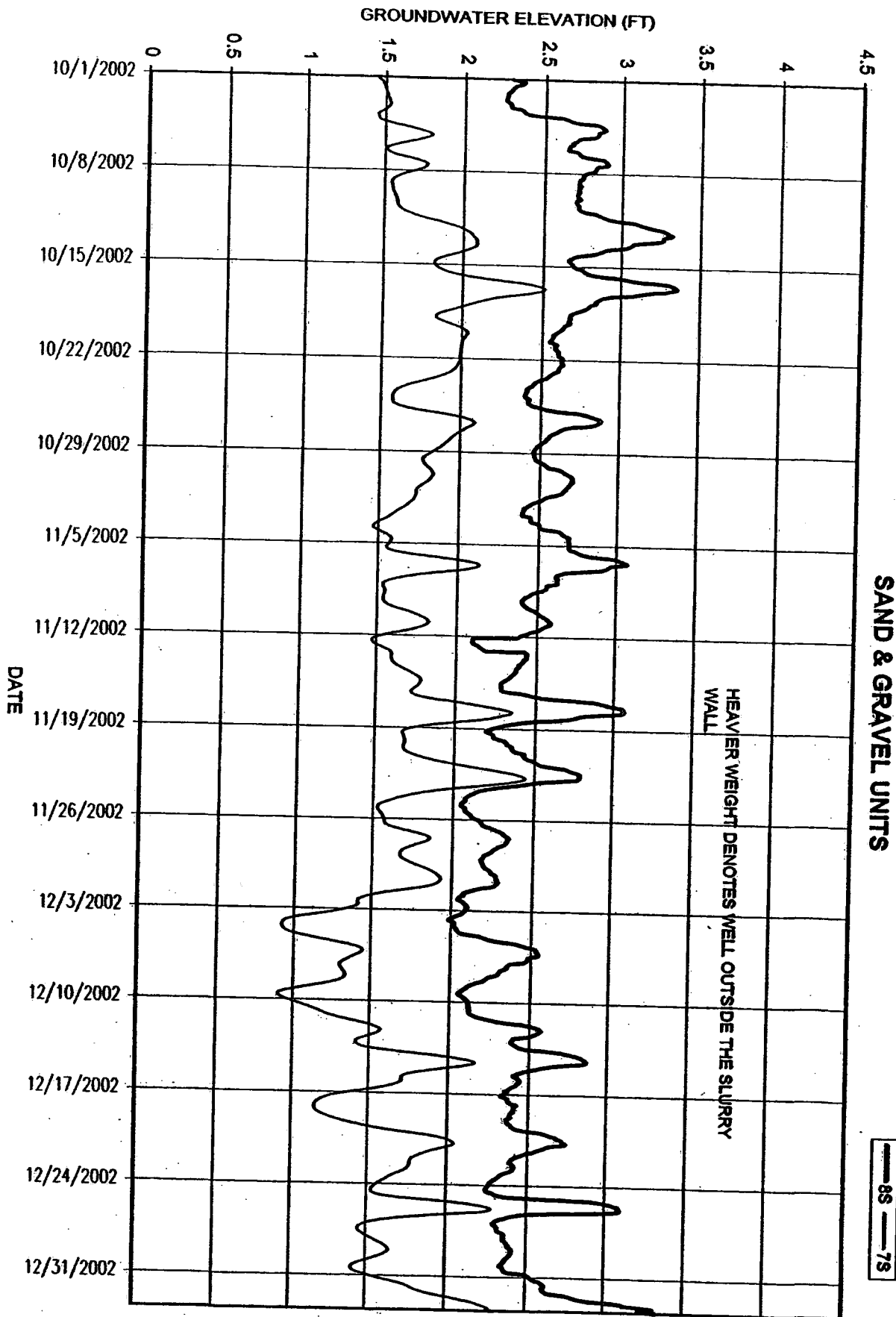
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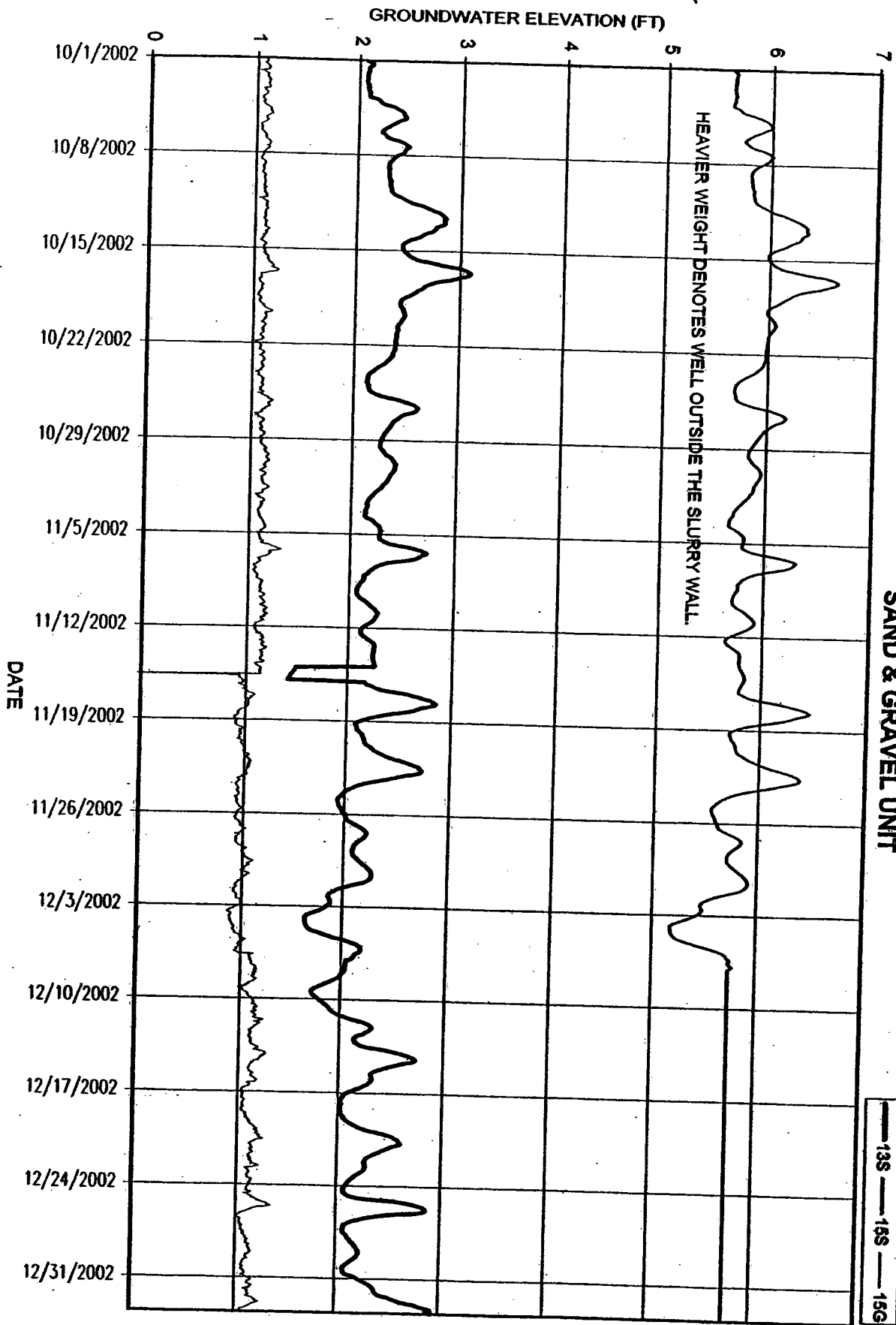
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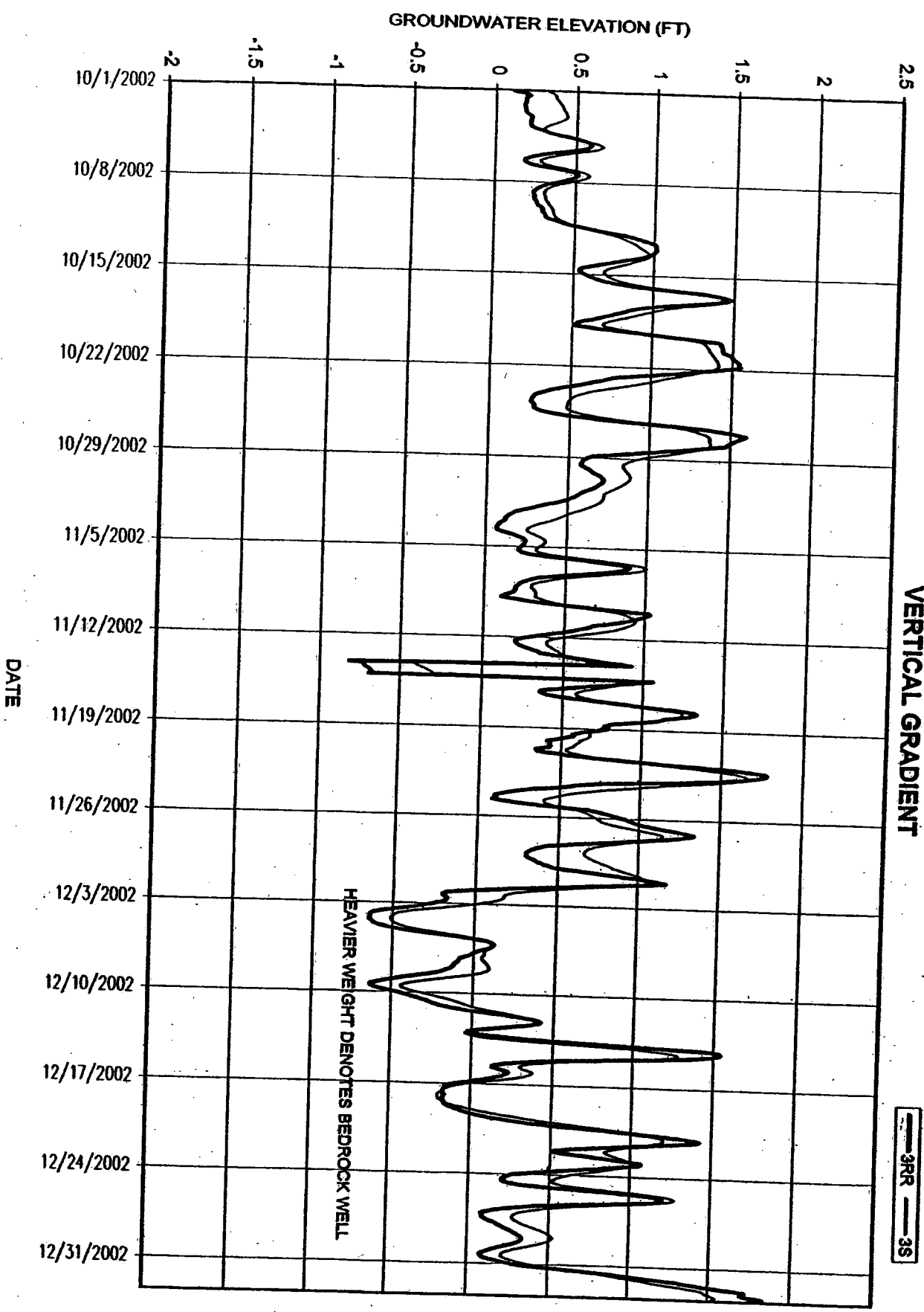
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #8 TRANSECT No.4 SAND & GRAVEL UNITS



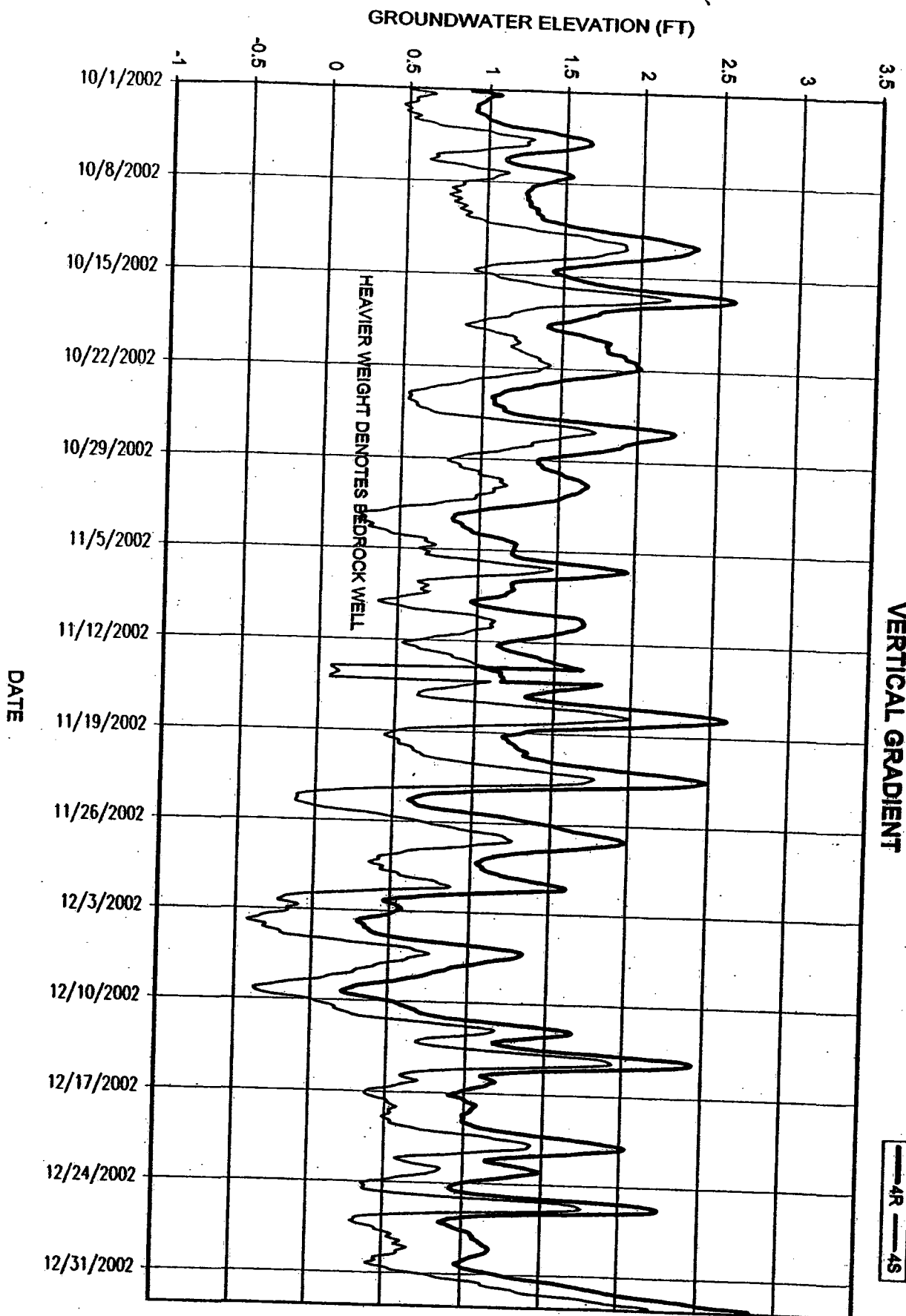
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #9 TRANSECT No.4 (OSA) SAND & GRAVEL UNIT



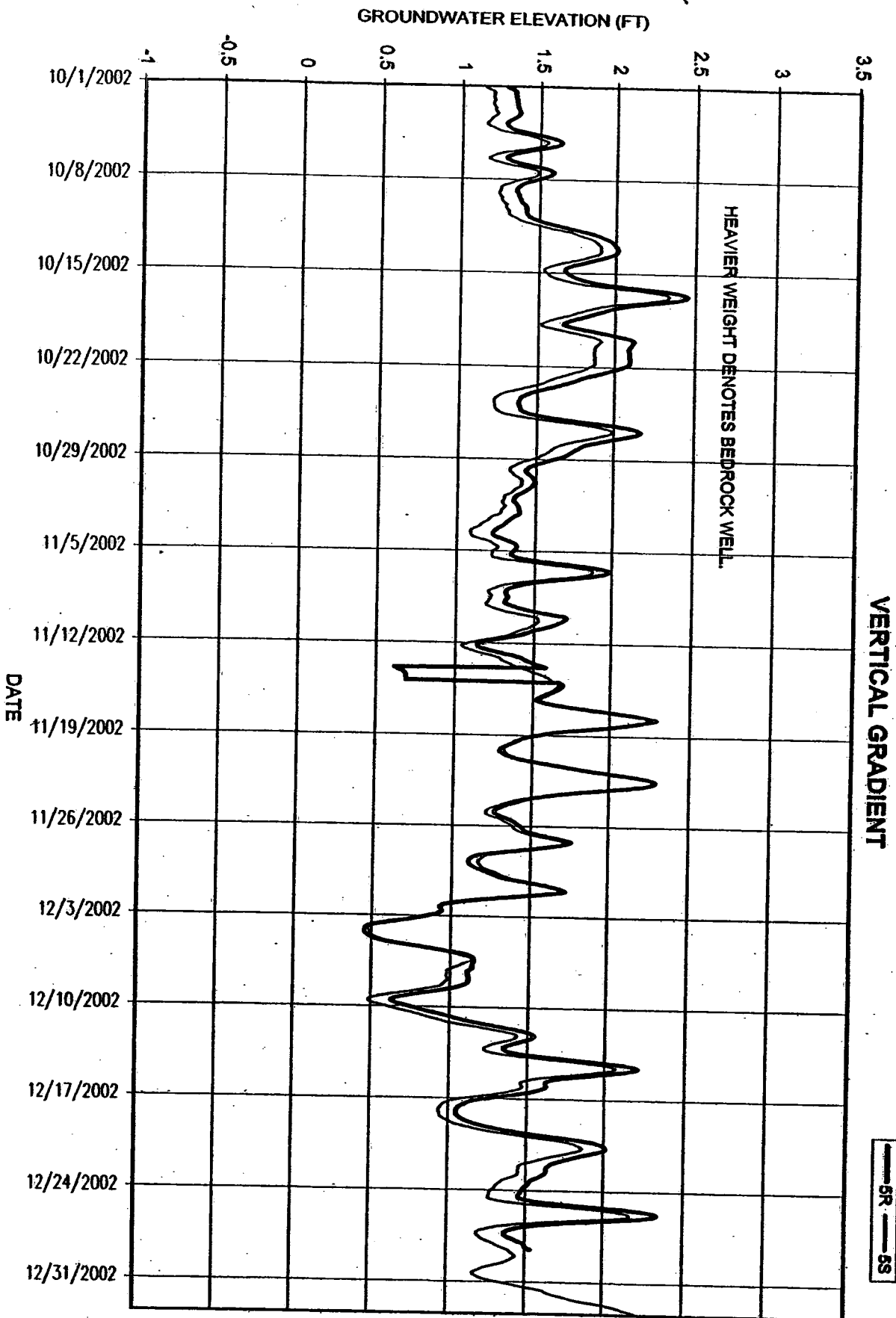
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #10 TRANSECT No.2 - INSIDE VERTICAL GRADIENT



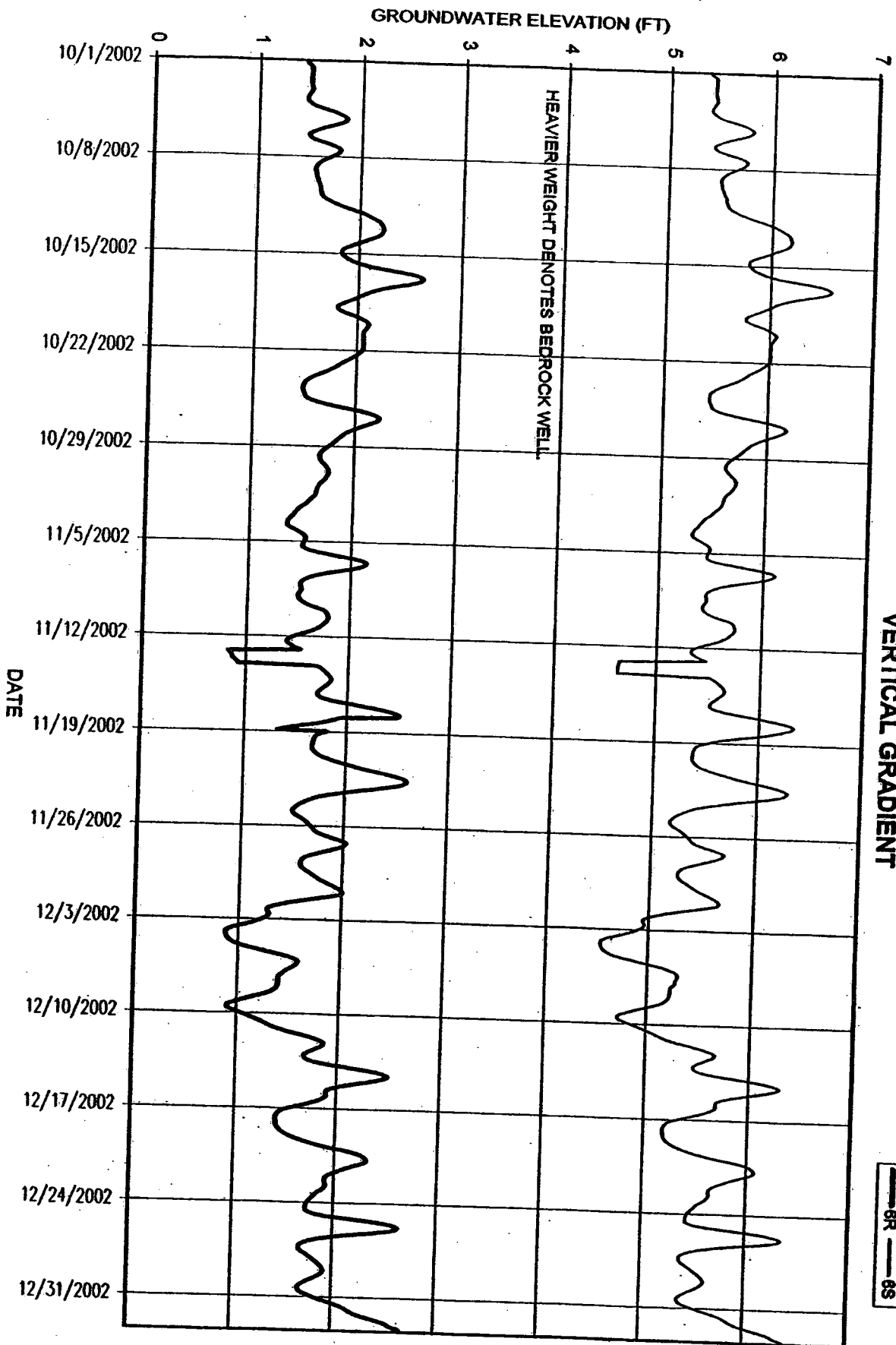
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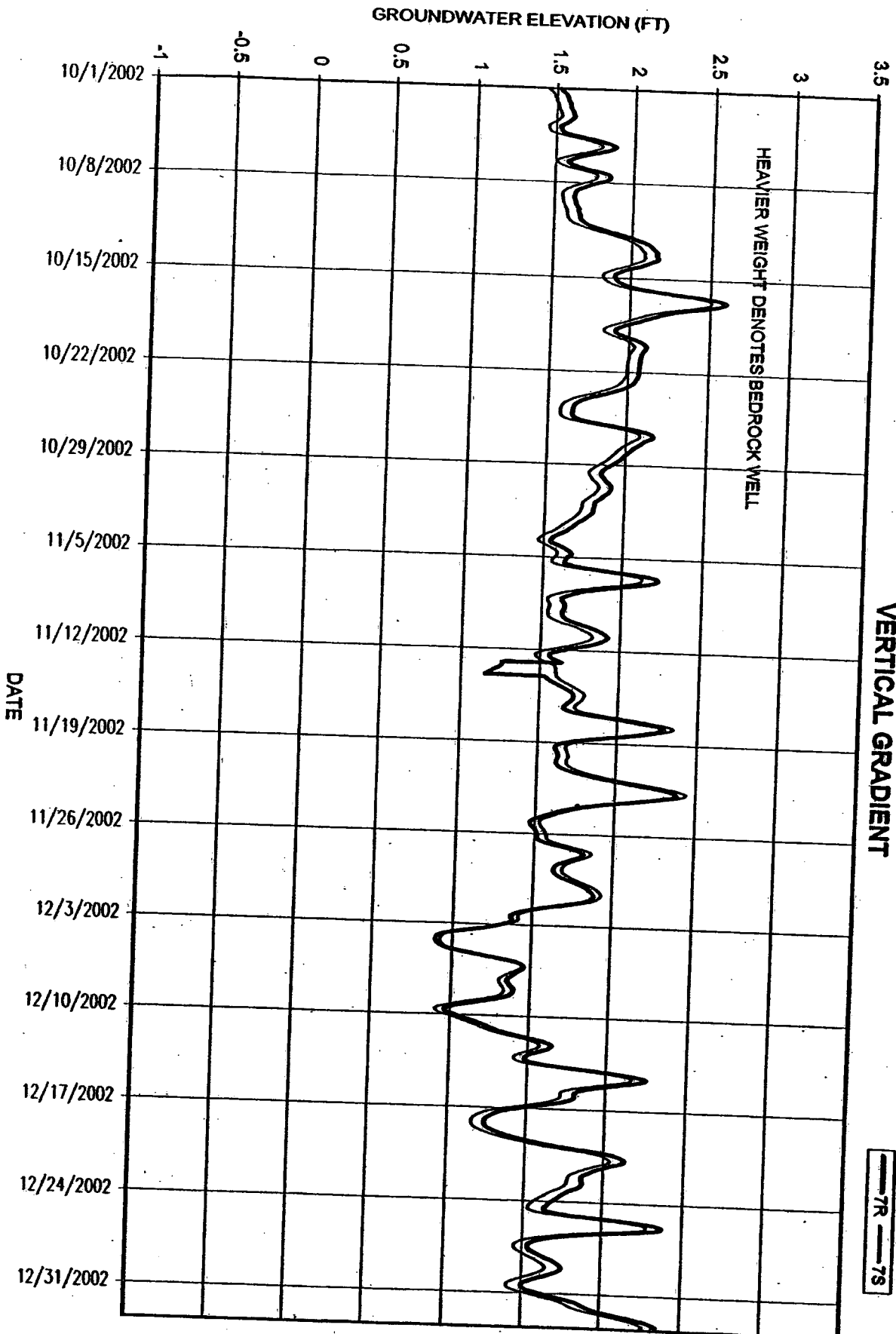
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #12 TRANSECT No.3 - INSIDE VERTICAL GRADIENT



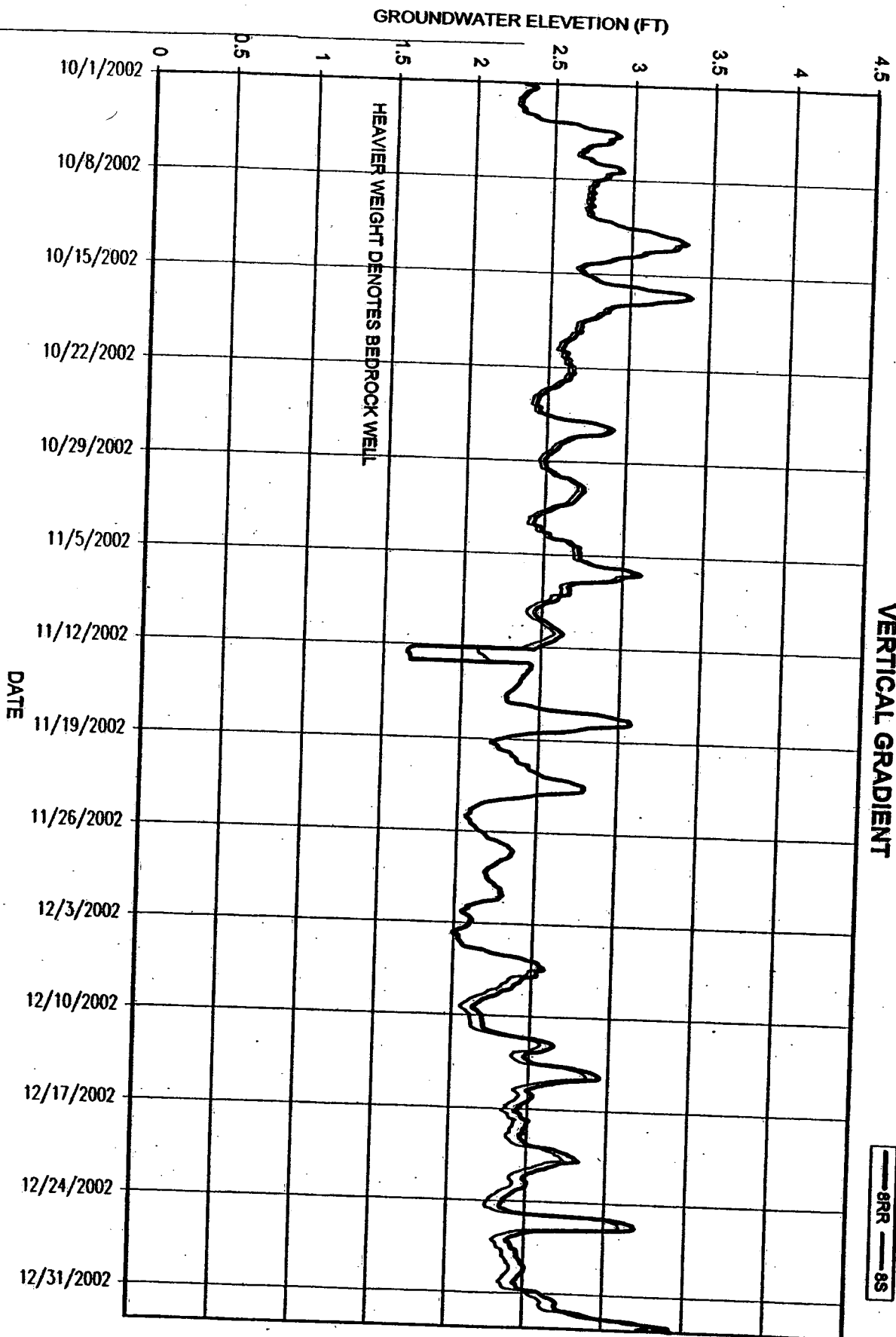
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #13 TRANSECT No.3 - OUTSIDE VERTICAL GRADIENT



KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #14 TRANSECT No.4- INSIDE VERTICAL GRADIENT



KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #15
 TRANSECT NO.4- OUTSIDE
 VERTICAL GRADIENT



APPENDIX B
MONTHLY HYDRAULIC EVALUATIONS



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January 13, 2003
Project 791186

Mr. Carl Januszkiewicz
Waste Management, Inc.
Kin-Buc Landfill Treatment Plant
383 Meadow Road
Edison, NJ 08817

Re: Hydraulic Monitoring for October 2002

Dear Mr. Januszkiewicz:

A site visit was completed on November 6, 2002 to download water level recorder data and obtain manual water level measurements. The following is an update of the hydraulic monitoring for the month of October 2002 at the Kin-Buc Landfill. This information is to be included in the quarterly report, which is to be submitted to the EPA by mid-February 2003.

The minimum, maximum, and average water elevations recorded at each well are included in Table 1. Table 2 shows the troll water elevations versus the manual water elevations. The continuous water level elevation data when compared with manual readings indicated that the miniTrolls and the SP 4000 Troll (in Well 15S) are functioning properly and are recording accurate data with the exception of Well 13G.

During the site visit on November 6, 2002, the miniTroll in Well 13G would not respond while trying to download the water level data. The EMCON/OWT, Inc. field technician was able to record the pressure reading from the miniTroll, but could not download the past month's data. The miniTroll (serial number 6171) was removed from the well on November 6, 2002 and sent to In-Situ, Inc. on November 7, 2002. After speaking with In-Situ, Inc. representative Glenn Carlson on November 22, 2002, it was noted that the memory chip malfunctioned and there were problems with the battery sensor. In-situ is attempting to manually recover the data from the miniTroll.

An SP 4000 Troll was temporarily installed in Well 13G during the site visit on December 6, 2002. This device will be utilized until the above-referenced miniTroll (serial number 6171) is repaired or replaced.

In addition, In-Situ, Inc. sent a replacement miniTroll (serial number 10275) for the miniTroll that had previously malfunctioned in Well 15S (serial number 7573) on June 23, 2002. The new miniTroll was subsequently installed in Well 15S during the site visit on December 6, 2002.

During the November 6, 2002 site visit, each of the miniTrolls was lifted from the well and visually inspected. Any discoloration or corrosion on the miniTroll or cable was noted as well as any error messages received while downloading water level data. This information was recorded in the field notes for each of the wells.

Hydrographs have been prepared for each of the transect locations and are enclosed for your reference as Attachment No. 1.

The water levels in wells on the outside of the slurry wall vary over the course of the day due to the tidal influence at the site. For clarity, Hydrograph Nos. 6 through 15 shows the average water level in the well over a 24-hour period (12 hours before, and 12 hours after).

Refuse

As defined in the Record of Decision (ROD) for OU-1, the performance objective for the refuse unit calls for the pumping of leachate to establish inward gradients across the slurry wall with the additional benefit of reducing downward flow into the underlying sand and gravel unit. Based on the hydrographs the following is presented.

Transect 1-Refuse (1G/2G)/Hydrograph No. 1 - Intragradients conditions were not observed during the month of October. The average monthly water elevation for October at Well 1G (inside) and Well 2G (outside) was 12.00 and 11.19 feet msl, respectively. Water level elevation measurements taken from Leachate Collection Cleanouts Nos. 14 through 16 are included in Table 3, and indicate that the leachate collection system is functioning properly. The fact that the leachate collection system is functioning properly suggests significant capture of leachate. The evaluation of the hydraulic conditions in the refuse at Transect 1 is provided in Attachment No. 2.

Transect 2-Refuse (3G/4G)/Hydrograph No. 2 - Intragradients conditions were maintained throughout the month. The average monthly water elevation for the month of October at Well 3G (inside) and Well 4G (outside) was 9.96 and 11.11 feet msl, respectively.

Transect 3-Refuse (5G/6G)/Hydrograph No. 3 - Intragradients conditions were maintained throughout the month. The average monthly water elevation for the month of October at Well 5G (inside) and Well 6G (outside) was 4.38 and 13.34 feet msl, respectively.

Transect 4-Refuse Oil Seeps Area (13G/15G)/Hydrograph No. 4 - No continuous water level data for Well 13G is available for the month of October due to the previously noted memory chip malfunction in the miniTroll (serial number 6171). The average monthly water elevation for the month of October at Well 15G (inside) and Well 13G

(outside) was 1.11 and 6.88 (taken from manual water level readings) feet msl, respectively.

Transect 5-Refuse (9G/10G)/Hydrograph No. 5 – Intragradients were maintained throughout the month. The average monthly water elevation for the month of October at Well 9G (inside) and Well 10G (outside) was 7.71 and 8.65 feet msl, respectively.

Sand and Gravel/Bedrock

For the sand and gravel unit, the performance objectives call for pumping of sand and gravel groundwater to prevent flow toward the slurry wall and to impose upward hydraulic gradients from the bedrock to the sand and gravel. An additional benefit would be the establishment of inward gradients across the slurry wall within the sand and gravel unit. The following is a description of the flow characteristics based on visual observation of the hydrographs:

Horizontal Flow

Transect 2-Sand and Gravel (3S/4S)/Hydrograph No. 6 – Intragradients were not consistently maintained for the month. The average monthly water elevations for the month of October at Well 3S (inside) and Well 4S (outside) was 0.79 and 1.09 feet msl, respectively.

Transect 3-Sand and Gravel (5S/6S)/Hydrograph No. 7 – Intragradients were maintained throughout the month of October. The average water elevations for Well 5S (inside) and Well 6S (outside) were 1.54 and 5.78 feet msl, respectively.

Transect 4-Sand and Gravel (7S/8S)/Hydrograph No. 8 – Intragradients were maintained throughout the month. The average monthly water elevation for the month of October at Well 7S (inside) and Well 8S (outside) was 1.82 and 2.69 feet msl, respectively.

Transect 4 Sand and Gravel Oil Seeps Area (13S/15S)/Hydrograph No. 9 – Intragradients were not evident during the month. The average monthly water elevation for the month of October at Well 15S (inside) and Well 13S (outside) was 5.95 and 2.41 feet msl, respectively. Water levels from Well 15G in the refuse unit are included on the hydrograph for comparison.

Vertical Flow-Inside Slurry Wall

Transect 2-Vertical Gradient (3S/3RR)-Inside/Hydrograph No.10 – Upward gradient conditions were not observed between the bedrock and overlying sand & gravel units inside the slurry wall for the month. The average monthly water

elevation for the month of October at Well 3S (sand & gravel) and Well 3RR (bedrock) was 0.79 and 0.70 feet msl, respectively.

Transect 3-Vertical Gradient (5R/5S)-Inside/Hydrograph No. 12 – Upward gradient conditions were observed throughout the month between the bedrock and overlying sand & gravel units inside the slurry wall. The average monthly water elevation for the month of October at Well 5S (sand & gravel) and Well 5R (bedrock) was 1.54 and 1.67 feet msl, respectively.

Transect 4-Vertical Gradient (7R/7S)-Inside/Hydrograph No. 14 – Slight upward gradient conditions were observed between the bedrock and overlying sand & gravel units inside the slurry wall throughout the month. The average monthly water elevation for the month of October at Well 7S (sand & gravel) and Well 7R (bedrock) was 1.82 and 1.89 feet msl, respectively. The difference in average monthly water elevations was less than 0.1 feet.

Vertical Flow-Outside Slurry Wall

Transect 2-Vertical Gradient (4S/4R)-Outside/Hydrograph No. 11 – Upward gradient conditions were observed between the bedrock and overlying sand & gravel units outside the slurry wall for the month. The average monthly water elevation for the month of October at Well 4S (sand & gravel) and 4R (bedrock) was 1.09 and 1.58 feet msl, respectively.

Transect 3-Vertical Gradient (6R/6S)-Outside/Hydrograph No. 13 – Upward gradient conditions were not observed between the bedrock and overlying sand & gravel units outside the slurry wall for the month. The average monthly water elevation for the month of October at Well 6S (sand & gravel) and Well 6R (bedrock) was 5.78 and 1.83 feet msl, respectively.

Transect 4-Vertical Gradient (8RR/8S)-Outside/Hydrograph No. 15 – Slight upward gradient conditions were observed between the bedrock and overlying sand & gravel units outside the slurry wall throughout the month. The average monthly water elevation for the month of October at both Well 8S (sand & gravel) and Well 8RR (bedrock) was 2.69 and 2.72 feet msl, respectively. The difference in average monthly water elevations was 0.3 feet.

An initial review of the hydrographs indicates that certain of the performance objectives associated with the sand and gravel and bedrock units may not be met, specifically associated with the uniform achievement of upward gradients from the bedrock to the overlying sand and gravel (e.g. Hydrographs 10 and 13), and inward gradients across the slurry wall within the sand and gravel (Hydrograph 6). However previous investigations performed at the site would indicate that complete control of OU-1 groundwater can be

achieved notwithstanding indications of downward flow from the sand and gravel to the bedrock, or outward flow across the slurry wall within the sand and gravel unit. This is based on the findings of the considerable pumping influence of the sand and gravel pumping wells, in particular S&G#2, in achieving hydraulic control at the site (see Groundwater Pumping Well Performance Evaluation Report, July 2000).

The influence of the pumping well can be demonstrated by a review of plan view groundwater contour map of the sand and gravel (Figure 1) and equipotential profiles and vector diagrams (Figures 2, 3, and 4) that have been prepared for a period of time when the vertical gradient between the sand and gravel and the bedrock was downward, and flow across the slurry wall within the sand and gravel unit was outward. For this evaluation, a snapshot of groundwater elevations from the monitoring wells and pumping wells was obtained for October 24, 2002. At this time S&G#2 was pumping at a rate of about 16 gallons per minute (gpm) while S&G#3 was pumping at a rate of 5 gpm for a total of 21 gpm or about 30,000 gallons per day. There was a downward vertical gradient observed between the sand and gravel and the bedrock inside the slurry wall at Transect No. 2, and outside the slurry wall at Transect 3 as evidenced by higher heads in the sand and gravel wells relative to bedrock wells. There was also a higher head within the sand and gravel inside the slurry wall relative to the sand and gravel outside the slurry wall at Transect No. 2.

Figures 1-4 incorporate the heads induced by pumping and show the considerable pumping influence of S&G#2. (The hydraulic head at S&G#3 is not included in these figures because the transducer in this well had malfunctioned during this time period). Specifically, groundwater flowing downward from the sand and gravel into the bedrock is subsequently induced toward the pumping well. This occurs both inside and outside of the slurry wall. Also, groundwater within the sand and gravel unit is induced toward the pumping well. The considerable pumping influence demonstrated at S&G#2, in conjunction with the fact that natural groundwater gradients in both the sand & gravel and bedrock flow predominantly towards the area of S&G#2, result in the complete capture of OU-1 groundwater at these pumping rates.

Groundwater and Leachate Collection

Based on data provided by U.S. Filter, the following volumes of groundwater and leachate were extracted from the sand & gravel wells and leachate collection system for the period from October 1 to October 31, 2002:

S&G No. 1 Groundwater	S&G No. 2 Groundwater	S&G No. 3 Groundwater	S&G No. 4 Groundwater	Leachate
0 gal.	609,084 gal.	199,765 gal.	14,274 gal.	41,831 gal.
0 gpd	19,648 gpd	6,444 gpd	460 gpd	1,349 gpd

For the period, a total of 823,123 gallons of groundwater was collected. The average daily groundwater extraction rate for all of the wells was 26,552 gpd. The extraction rate from S&G No. 2 was 19,648 gpd, from S&G No. 3 it was 6,444 gpd, and the extraction rate from S&G No. 4 was 460 gpd. The leachate extraction rate of 1,349 gpd was slightly below the recommended rate of 1,500 gpd.

CONCLUSIONS

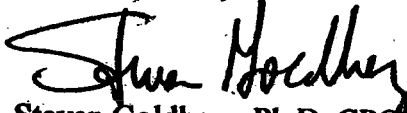
- Intragradiant conditions were maintained in the refuse unit at Transects 2, 3, 4, and 5.
- Intragradiant conditions were not indicated by the monitoring wells at Transect 1, although levels in the leachate collection system indicate intragradiant conditions are present at this location.
- Hydraulic control was maintained within OU-1 based on the analysis of the significant influence of S&G#2 in acting as a hydraulic sink for sand and gravel and bedrock groundwater. Groundwater flow in the sand and gravel and bedrock is ultimately captured by the pumping well resulting in overall containment of groundwater in OU-1.
- A hydraulic evaluation consistent with what has been presented herein will be performed for November 2002.
- In view of the analysis presented herein, it is recommended that the combined groundwater pumping rates in the sand and gravel be maintained at 15,000 gpd with S&G#2 and S&G#3 pumping at 10,000 gpd and 5,000 gpd, respectively. These lower pumping rates will be evaluated to confirm continued hydraulic control of OU-1 groundwater.

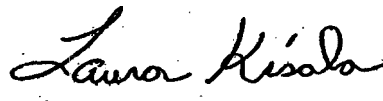
Mr. Carl Januszkiewicz
January 13, 2003
Page 7

Project 791186

We trust you find this information useful. If you have any questions, please do not hesitate to contact us.

EMCON/OWT, INC.


Steven Goldberg, Ph.D, CPG
Senior Hydrogeologist


Laura Kisala
Environmental Scientist

Attachments

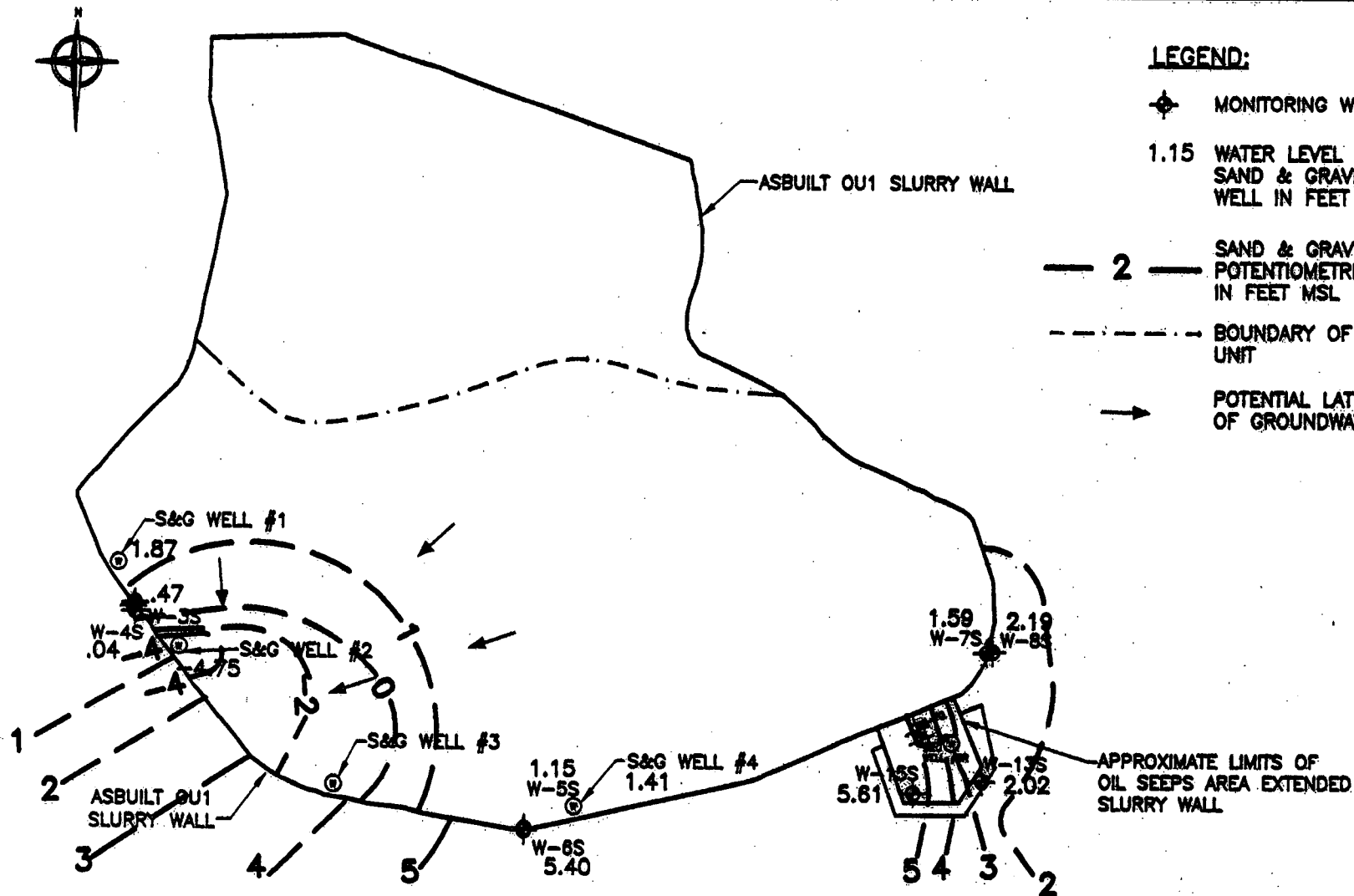
cc: Glenn Grieb, US Filter

LEGEND:
 MONITORING WELL

 1.15 WATER LEVEL ELEVATION IN
 SAND & GRAVEL MONITORING
 WELL IN FEET MSL

 — 2 — SAND & GRAVEL
 POTENTIOMETRIC SURFACE
 IN FEET MSL

 - - - - - BOUNDARY OF SAND & GRAVEL
 UNIT

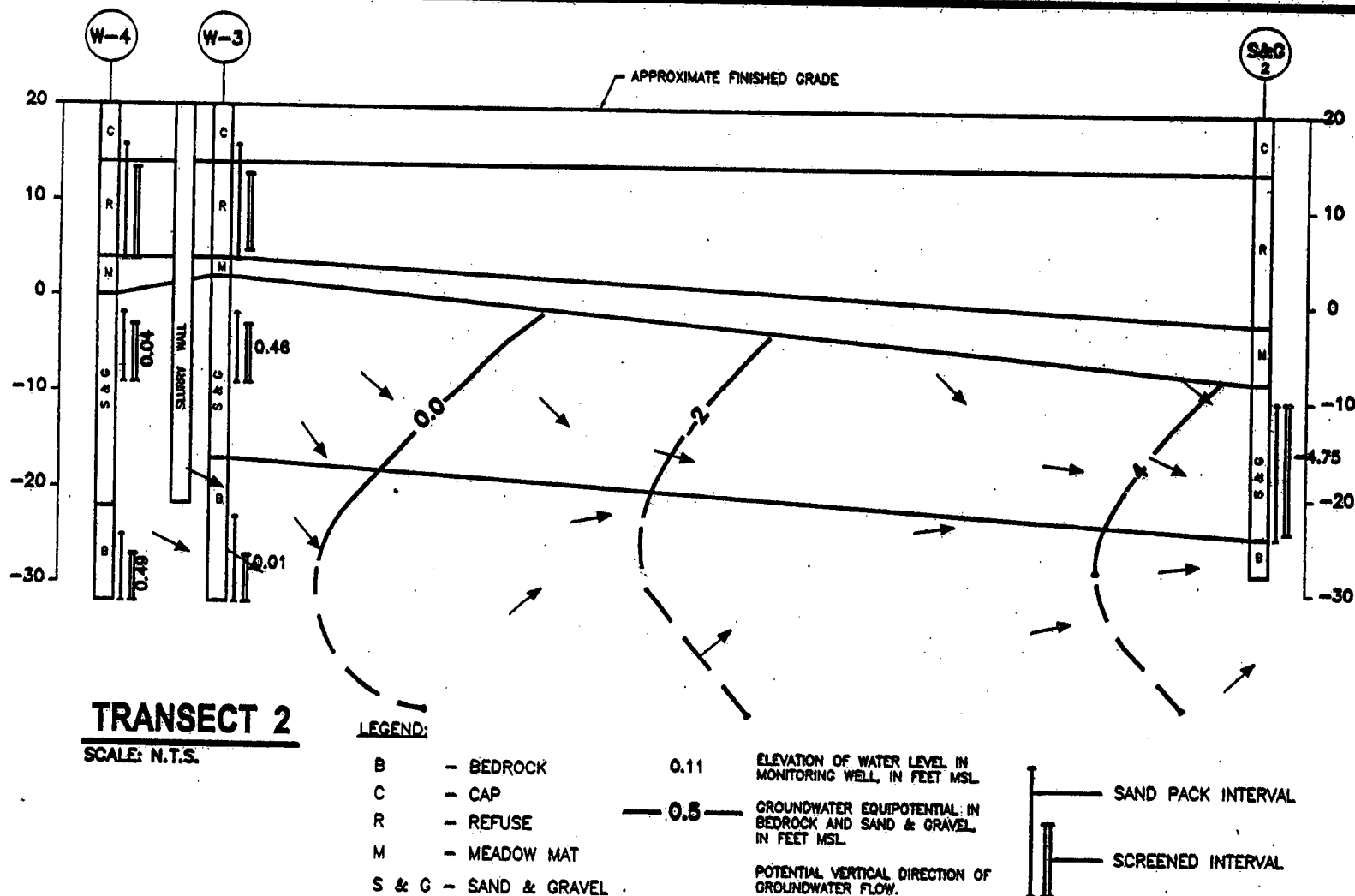
 POTENTIAL LATERAL DIRECTION
 OF GROUNDWATER FLOW


0 400 800
 SCALE IN FEET

DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO.
 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY

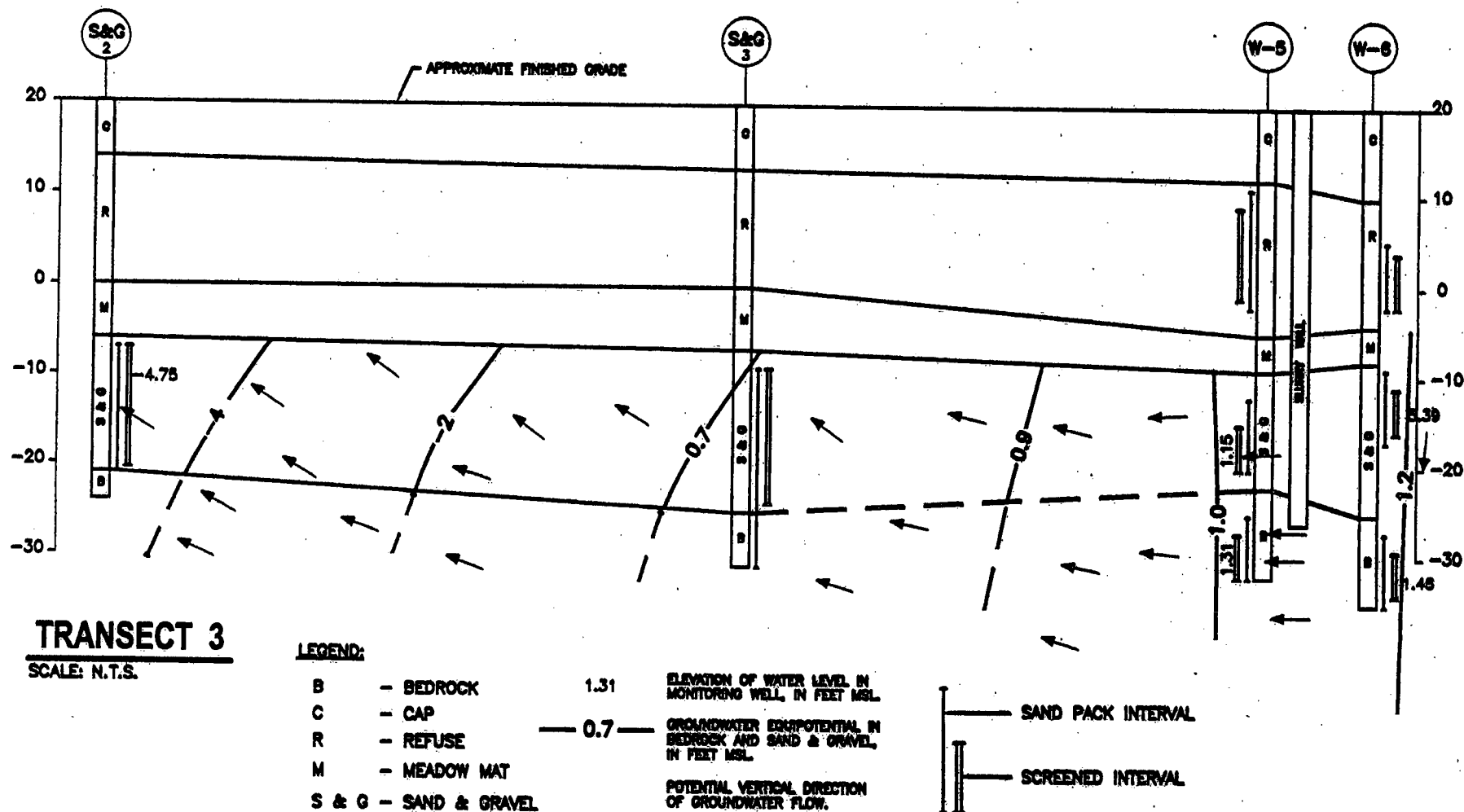
FIGURE 1
SAND & GRAVEL UNIT
POTENTIOMETRIC SURFACE



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO.
 791186

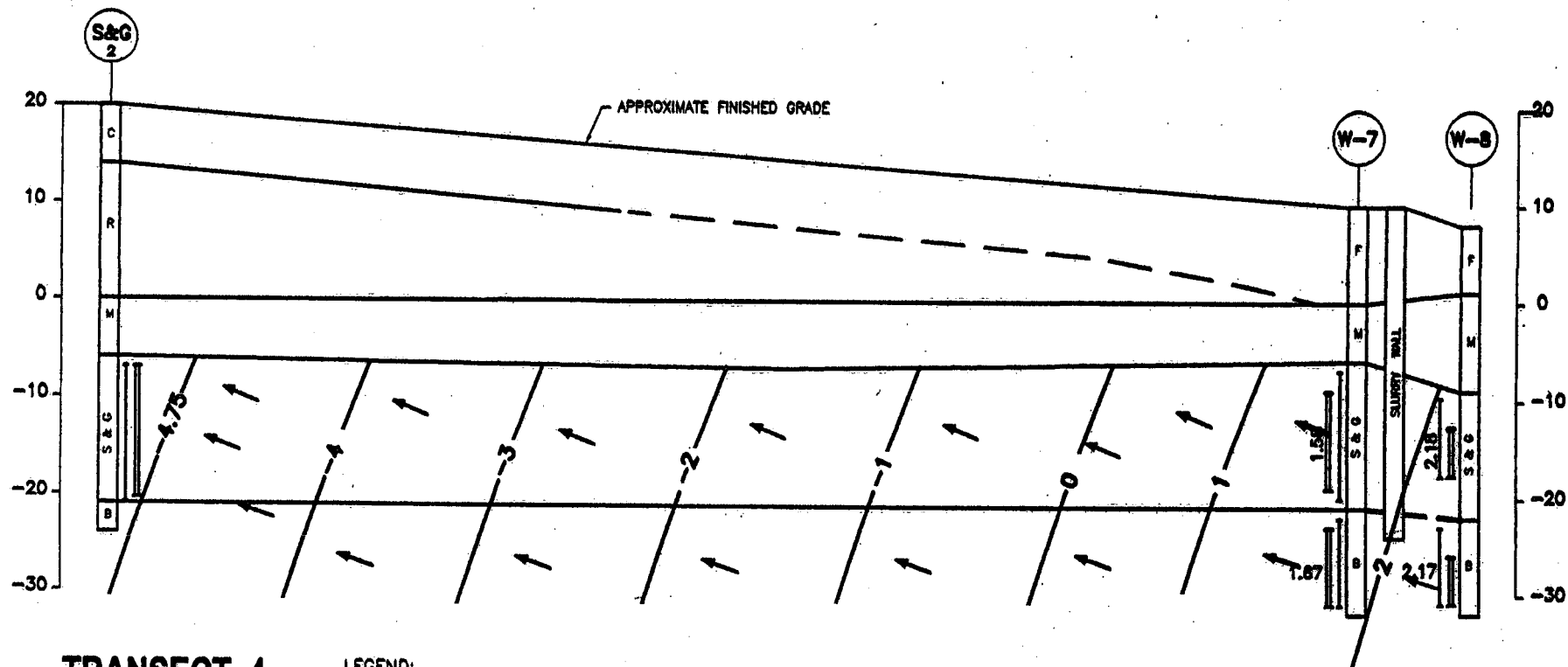
KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY

FIGURE 2
 TRANSECT 2
 HYDROGEOLOGIC CROSS SECTION ANALYSIS



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO.
 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
 FIGURE 3
 TRANSECT 3
 HYDROGEOLOGIC CROSS SECTION ANALYSIS



TRANSECT 4

SCALE: N.T.S.

LEGEND:

- B - BEDROCK
- C - CAP
- R - REFUSE
- M - MEADOW MAT
- S & G - SAND & GRAVEL

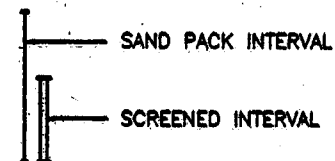
2.17

ELEVATION OF WATER LEVEL IN MONITORING WELL, IN FEET MSL.

-2-

GROUNDWATER EQUIPOTENTIAL IN BEDROCK AND SAND & GRAVEL, IN FEET MSL.

POTENTIAL VERTICAL DIRECTION OF GROUNDWATER FLOW.



DATE _____
DWN _____
APP _____
REV _____
PROJECT NO. 791186

KIN-BUC LANDFILL
EDISON TOWNSHIP, EDISON, NEW JERSEY
MIDDLESEX COUNTY, NEW JERSEY
FIGURE 4
TRANSECT 4
HYDROGEOLOGIC CROSS SECTION ANALYSIS

Table 1
KinBuc Landfill Operable Units 1 and 2
Continuous Hydraulic Monitoring Results
2002 Minimum/Maximum/Average Water Elevations

Inside Slurry Wall					Outside Slurry Wall				
Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)	Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)
W-1G	October	11.86	12.18	12.00	W-2G	October	10.26	12.48	11.19
W-3G	October	9.77	10.27	9.96	W-4G	October	10.78	11.57	11.11
W-3S	October	0.14	1.83	0.79	W-4S	October	-0.37	2.84	1.09
W-5G	October	4.18	4.93	4.38	W-6G	October	12.77	14.32	13.34
W-5S	October	0.94	2.66	1.54	W-6S	October	5.16	6.95	5.78
W-7S	October	1.35	2.85	1.82	W-8S	October	1.97	4.68	2.69
W-15S	October	5.48	7.11	5.95	W-13S	October	1.88	3.78	2.41
W-15G	October	1.02	1.24	1.11	W-13G	October	NA (1)	NA (1)	6.88 (2)
W-9G	October	7.54	7.87	7.71	W-10G	October	8.57	8.72	8.65
W-3RR	October	-0.37	1.97	0.70	W-4R	October	-0.06	3.37	1.58
W-5R	October	1.08	2.78	1.87	W-6R	October	1.27	2.93	1.83
W-7R	October	1.43	2.92	1.89	W-8RR	October	1.99	4.72	2.72

Notes:

1. Troll malfunctioned, data was not collected.
2. Water elevation calculated from manual water levels.

Table 2
KinBuc Landfill Operable Unit 1
October 2002
Troll Water Elevations vs. Manual Water Elevations

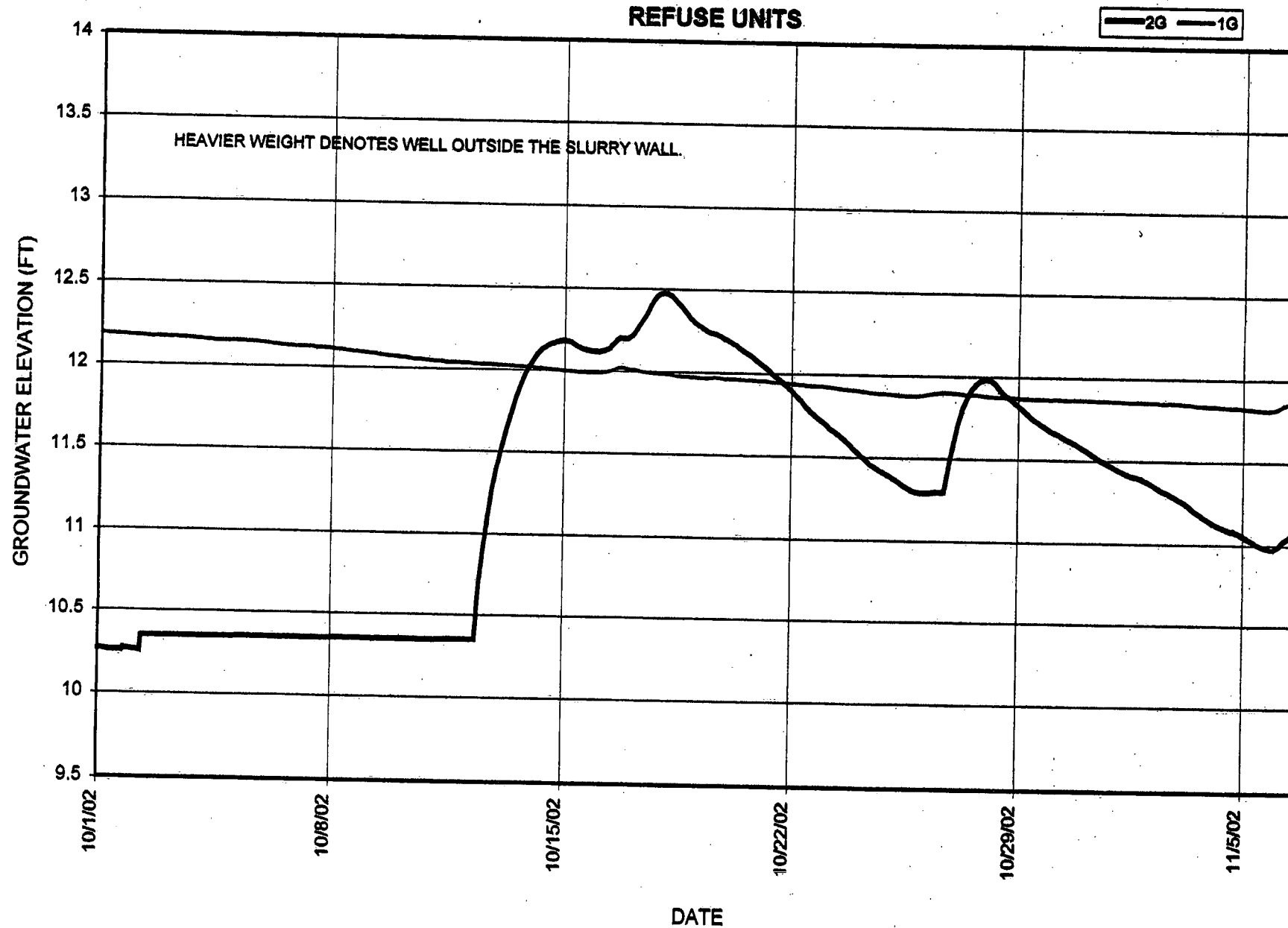
OU 1 Well ID	November 6, 2002		
	Troll	Manual	Difference
W-1G	11.87	11.81	0.06
W-2G	11.27	11.27	0.00
W-3G	9.34	9.29	0.05
W-3S	1.28	1.31	-0.03
W-3RR	1.50	1.50	0.00
W-4G	11.43	11.42	0.01
W-4S	2.56	2.05	0.51
W-4R	2.62	2.56	0.06
W-5G	10.38	10.40	-0.02
W-5S	2.12	2.16	-0.04
W-5R	1.92	1.96	-0.04
W-6G	13.84	13.82	0.02
W-6S	2.38	2.43	-0.05
W-6R	2.44	2.44	0.00
W-7S	2.29	2.31	-0.02
W-7R	2.43	2.45	-0.02
W-8S	2.61	2.63	-0.02
W-8RR	2.62	2.60	0.02
W-9G	7.87	7.89	-0.02
W-10G	8.70	8.70	0.00
W-13G	6.89	6.88	0.01
W-13S	2.55	2.54	0.01
W-15G	1.61	1.61	0.00
W-15S	2.72	2.69	0.03

Table 3
Kin-Buc Landfill
Leachate Cleanout Monitoring
2002

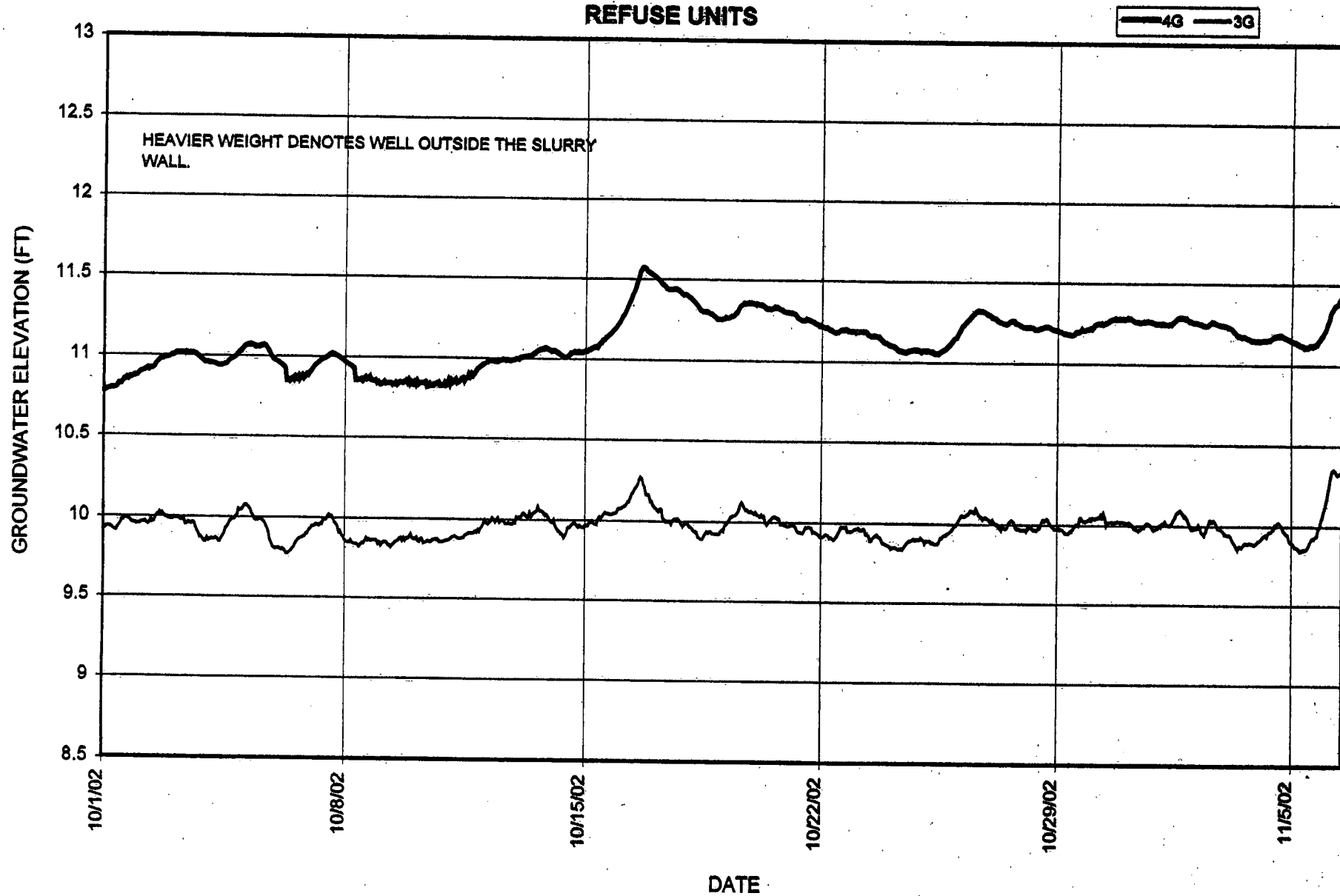
Cleanout location	14N		14E		15N		15E		16N		16E	
Elevation @ Sea Level	22.87		22.77		26.51		26.51		31.36		31.32	
	depth to water	elevation	depth to water	elevation	depth to water	elevation	depth to water	elevation	depth to water	elevation	depth to water	elevation
Elevation Average		10.11		10.08		9.87		9.96		na		na
DATE												
12/10/01	12.5	10.37	12.42	10.35	16.31	10.20	16.33	10.18	dry	na	dry	na
1/3/02	12.37	10.50	12.31	10.46	16.21	10.30	16.22	10.29	dry	na	dry	na
2/13/02	12.70	10.17	12.63	10.14	16.57	9.94	16.62	9.89	dry	na	dry	na
3/27/02	12.61	10.26	12.55	10.22	16.52	9.99	16.47	10.04	dry	na	dry	na
4/19/02	12.75	10.12	12.68	10.09	16.64	9.87	16.61	9.90	dry	na	dry	na
5/3/02	13.03	9.84	12.96	9.81	16.97	9.54	16.94	9.57	dry	na	dry	na
6/5/02	13.04	9.83	12.97	9.80	16.63	9.88	16.95	9.56	dry	na	dry	na
7/8/02	12.86	10.01	12.79	9.98	16.77	9.74	16.72	9.79	dry	na	dry	na
8/2/02	12.86	10.01	12.79	9.98	16.8	9.71	15.73	10.78	dry	na	dry	na
9/5/02	12.86	10.01	12.78	9.99	16.77	9.74	16.75	9.76	dry	na	dry	na
9/26/02	12.94	9.93	12.85	9.92	16.85	9.66	16.83	9.68	dry	na	dry	na
11/6/02	12.64	10.23	12.58	10.19	16.59	9.92	16.48	10.03	dry	na	dry	na

ATTACHMENT 1

KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #1
TRANSECT No. 1
REFUSE UNITS

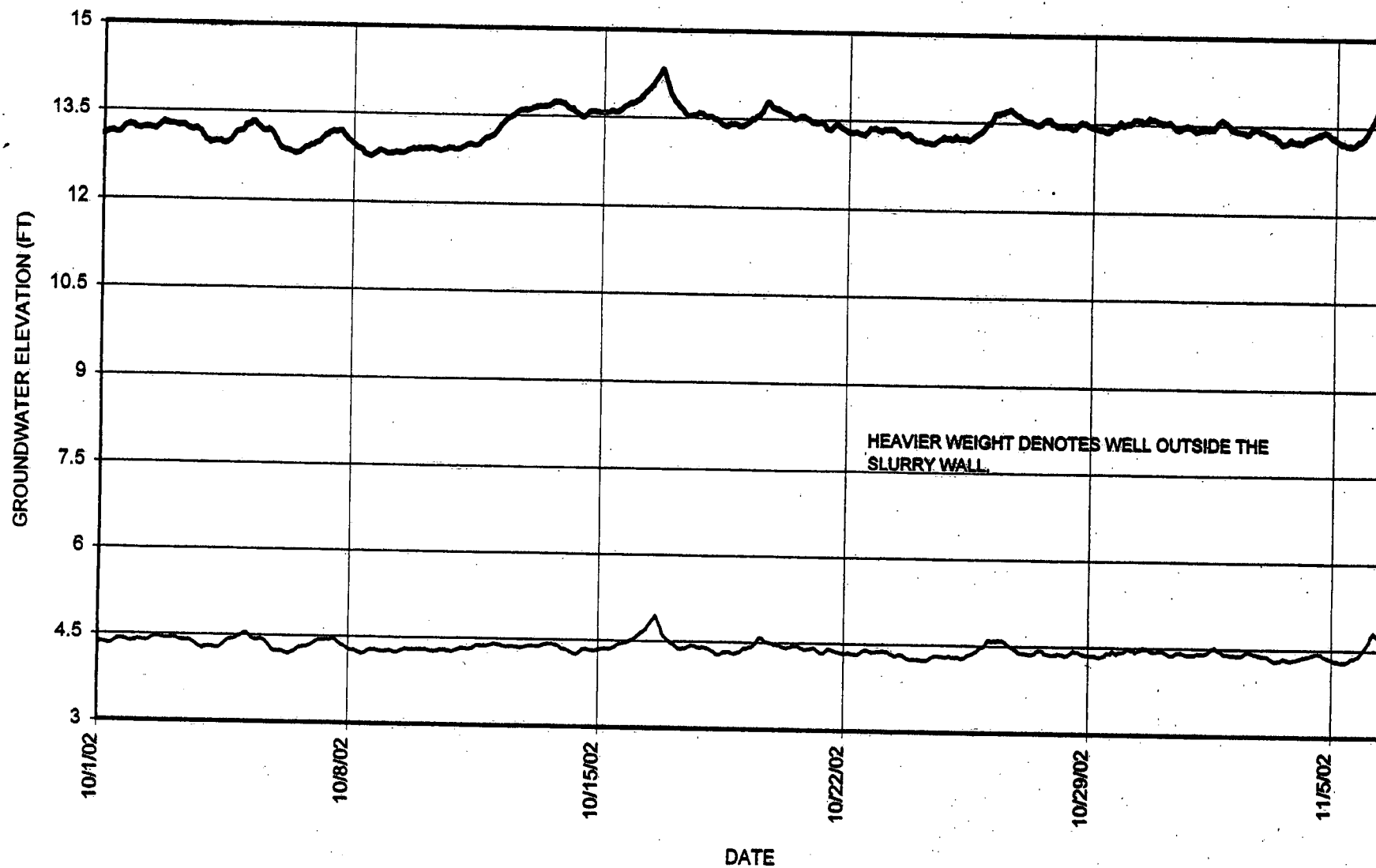


KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #2
TRANSECT No.2
REFUSE UNITS

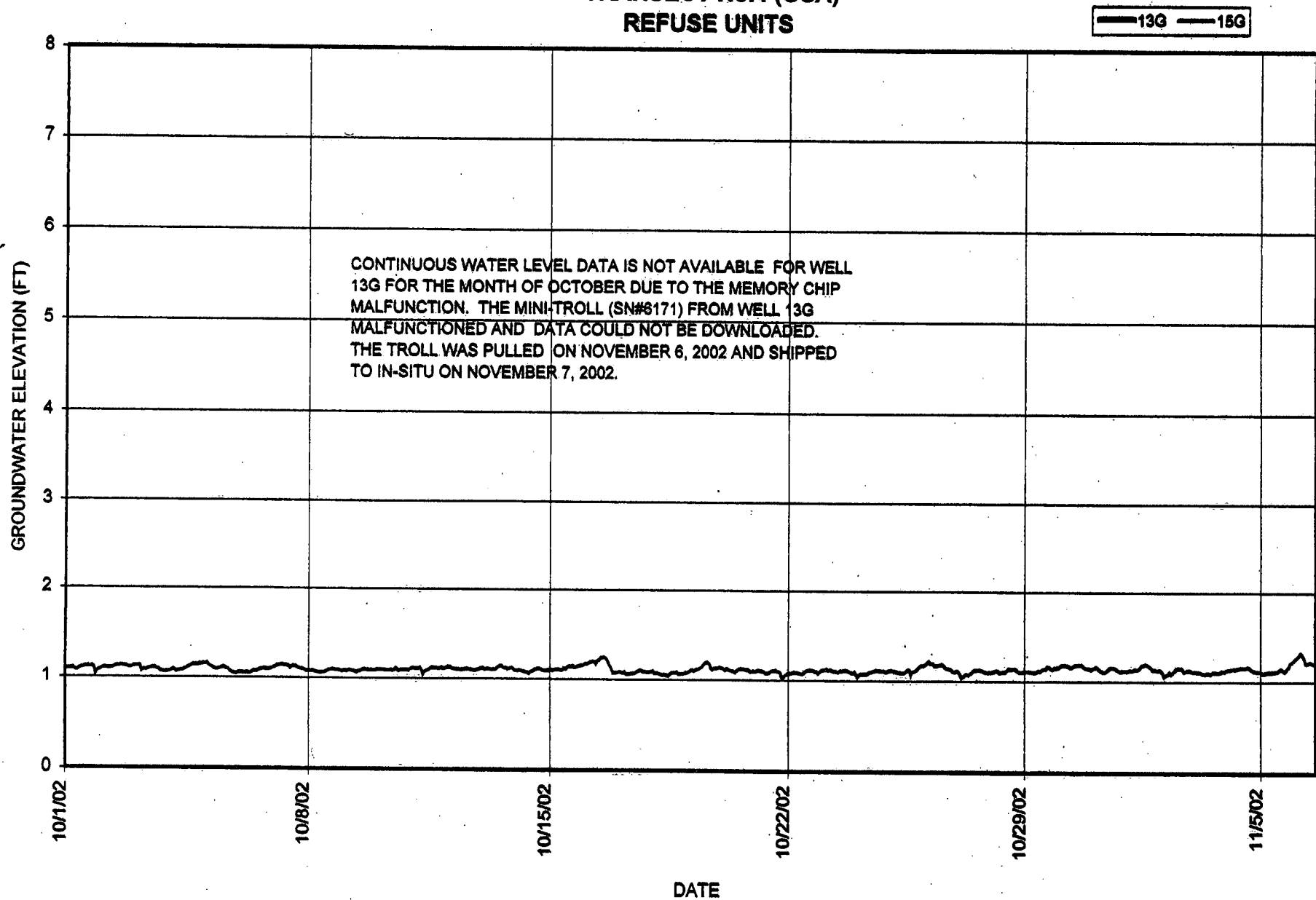


KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH # 3
TRANSECT No.3
REFUSE UNITS

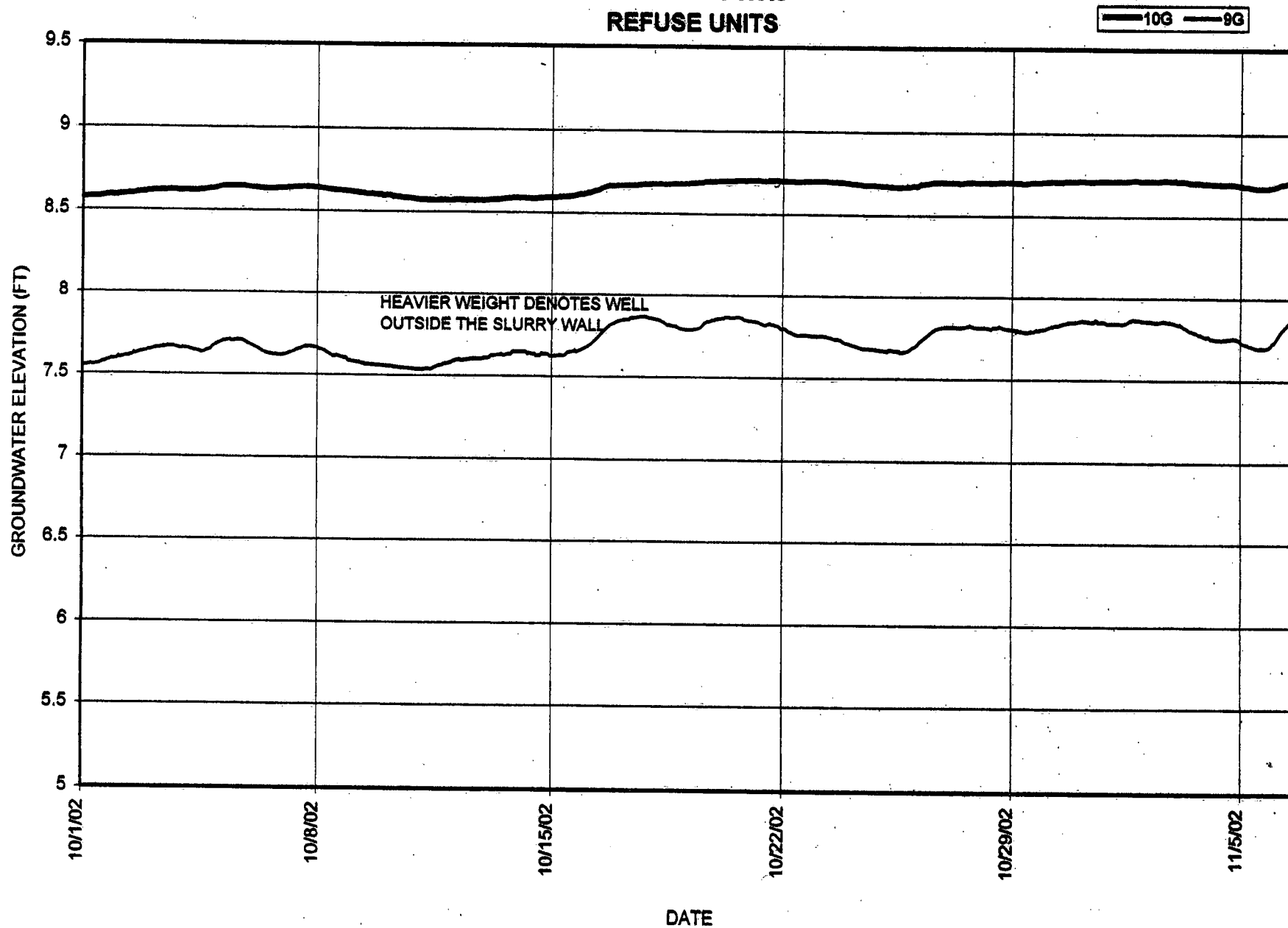
— 5G — 6G



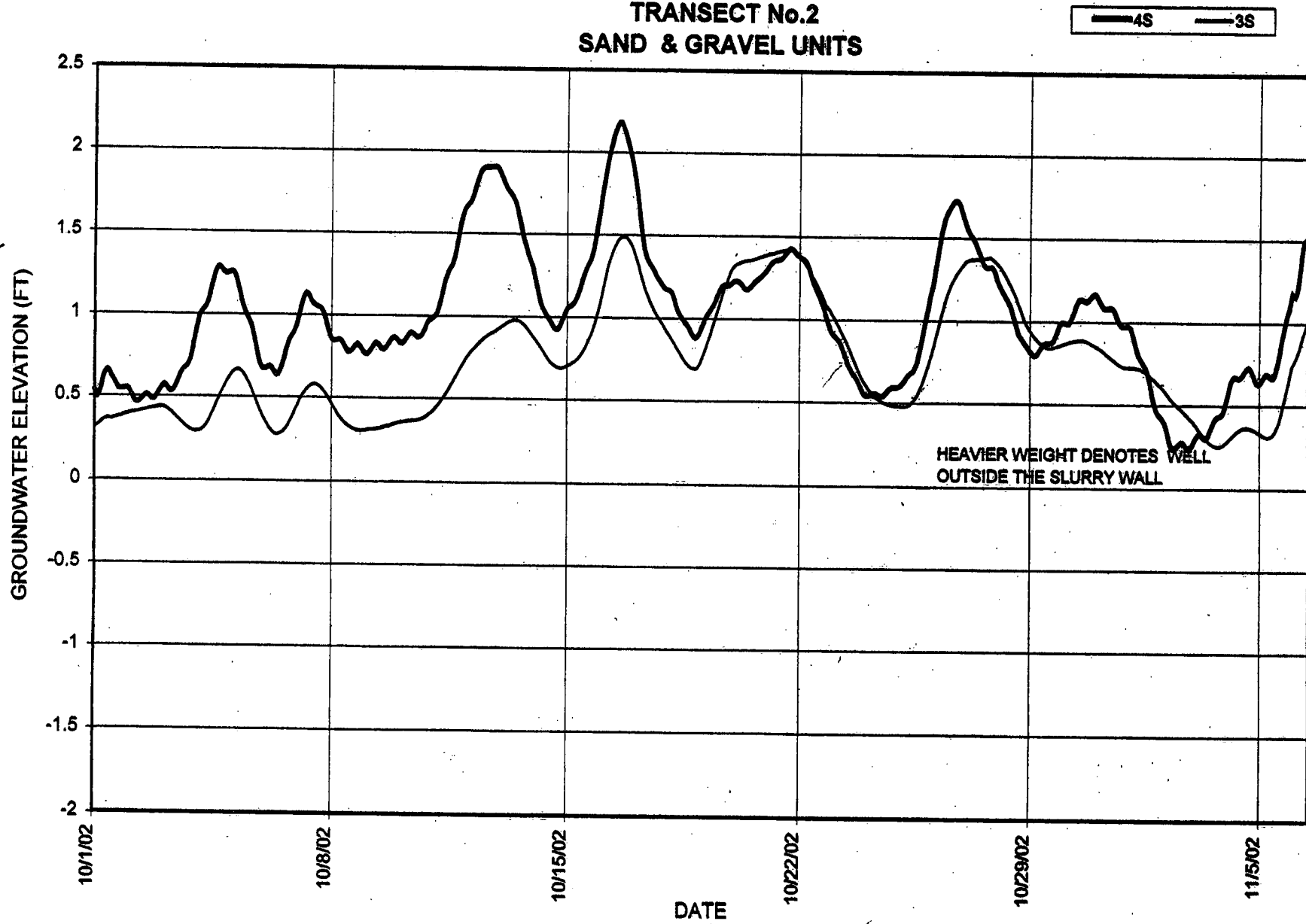
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #4
TRANSECT No.4 (OSA)
REFUSE UNITS



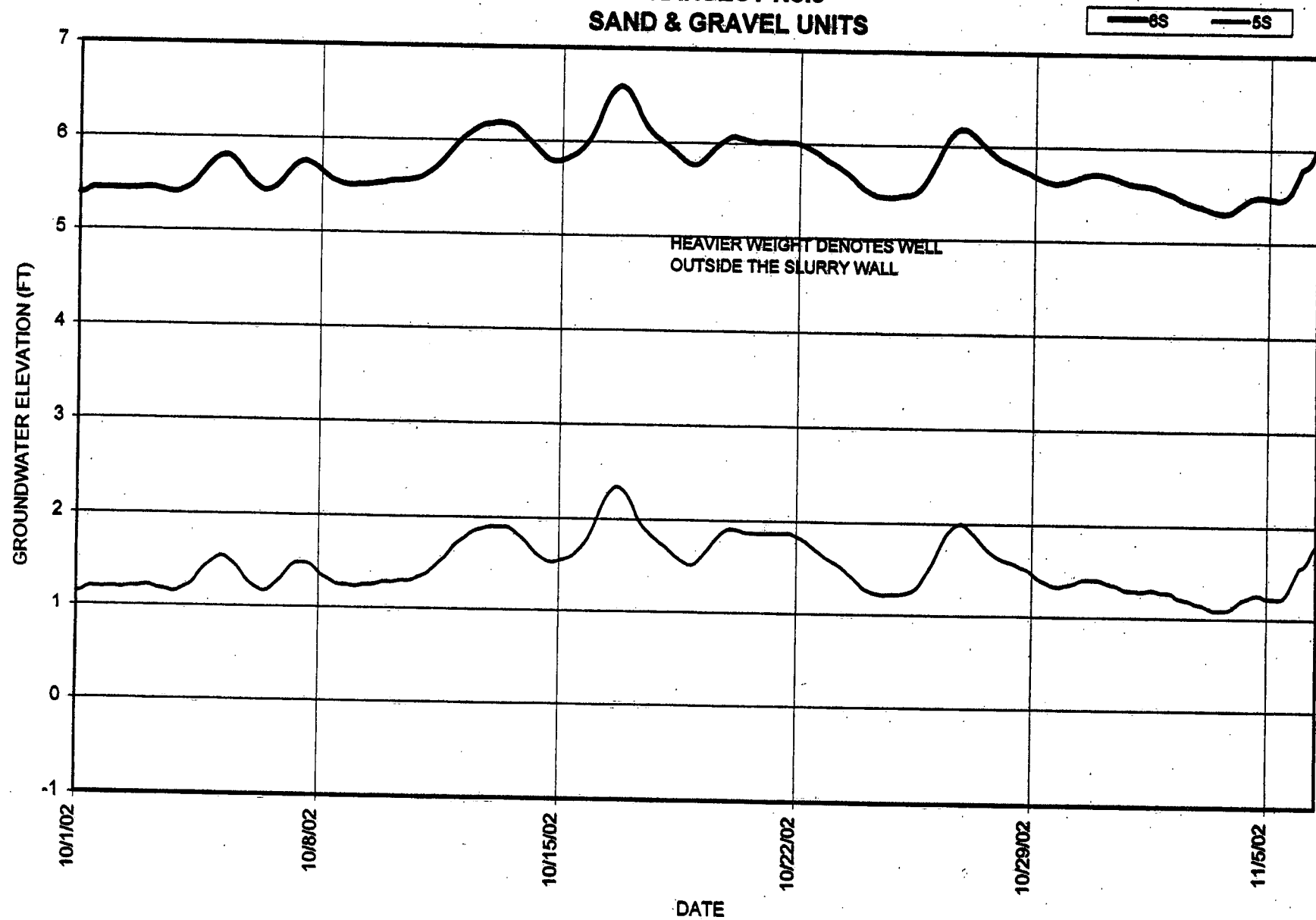
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TRANSECT No.5
REFUSE UNITS



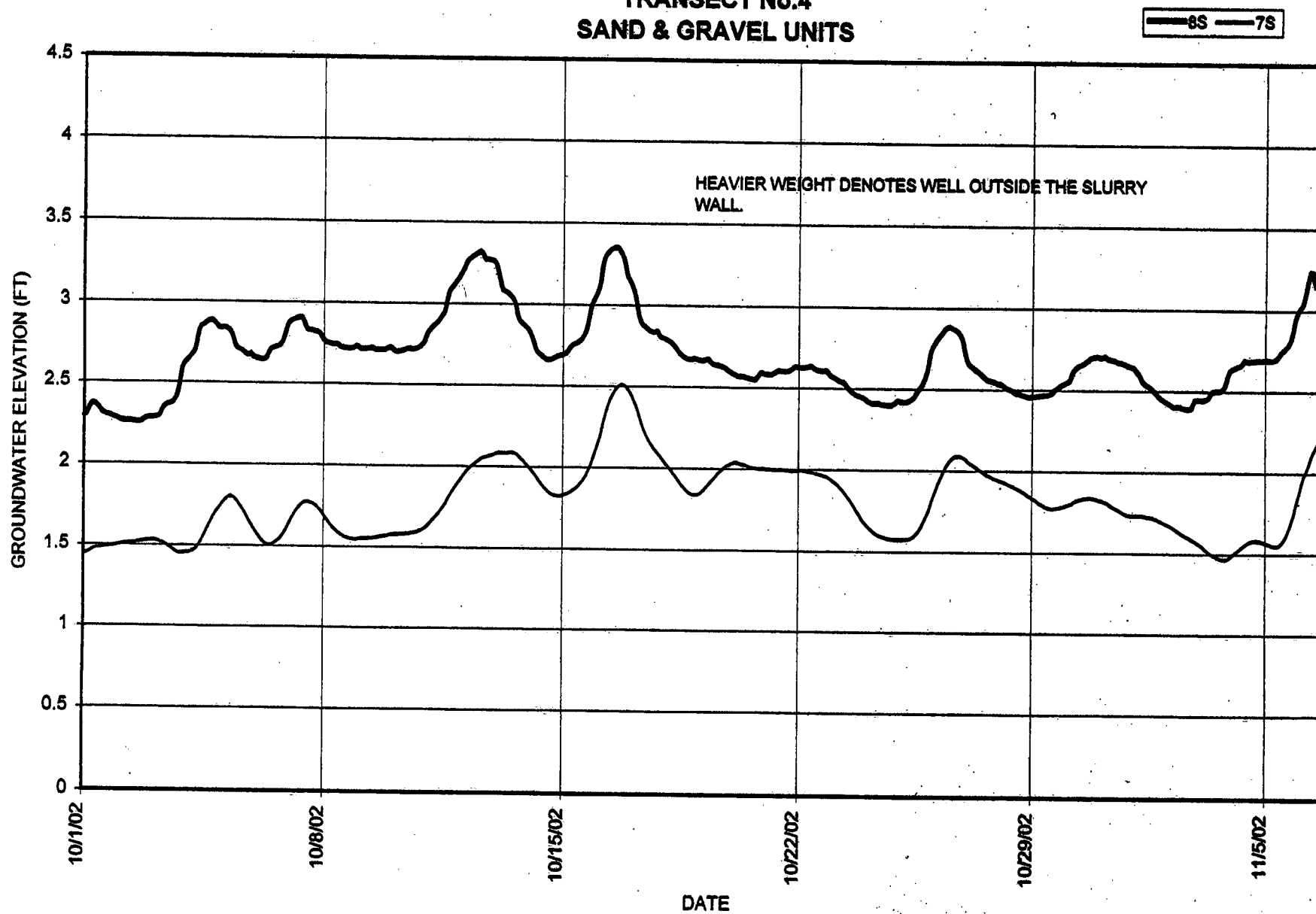
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #6
TRANSECT No.2
SAND & GRAVEL UNITS



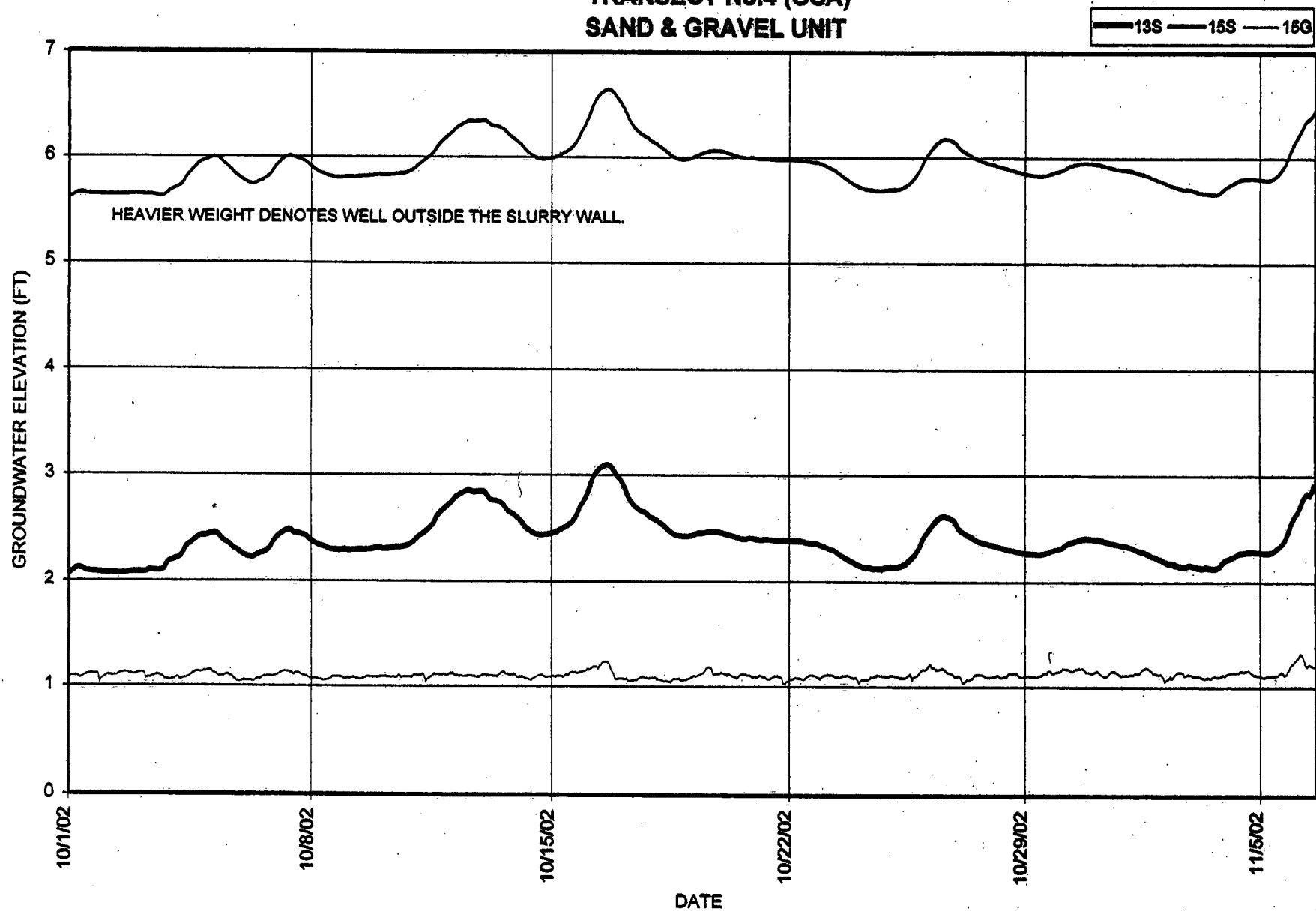
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #7
TRANSECT No.3
SAND & GRAVEL UNITS



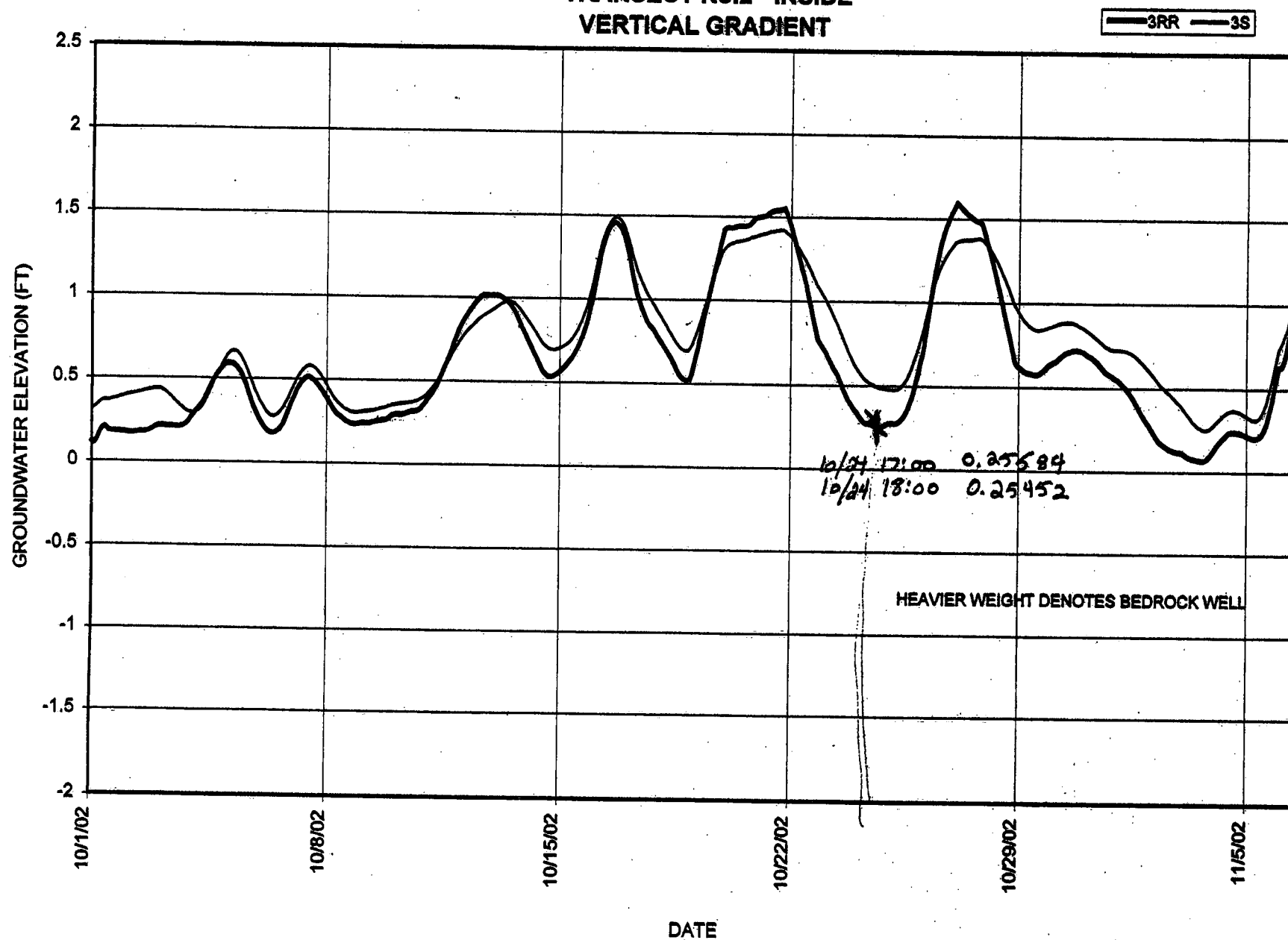
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #8
TRANSECT No.4
SAND & GRAVEL UNITS



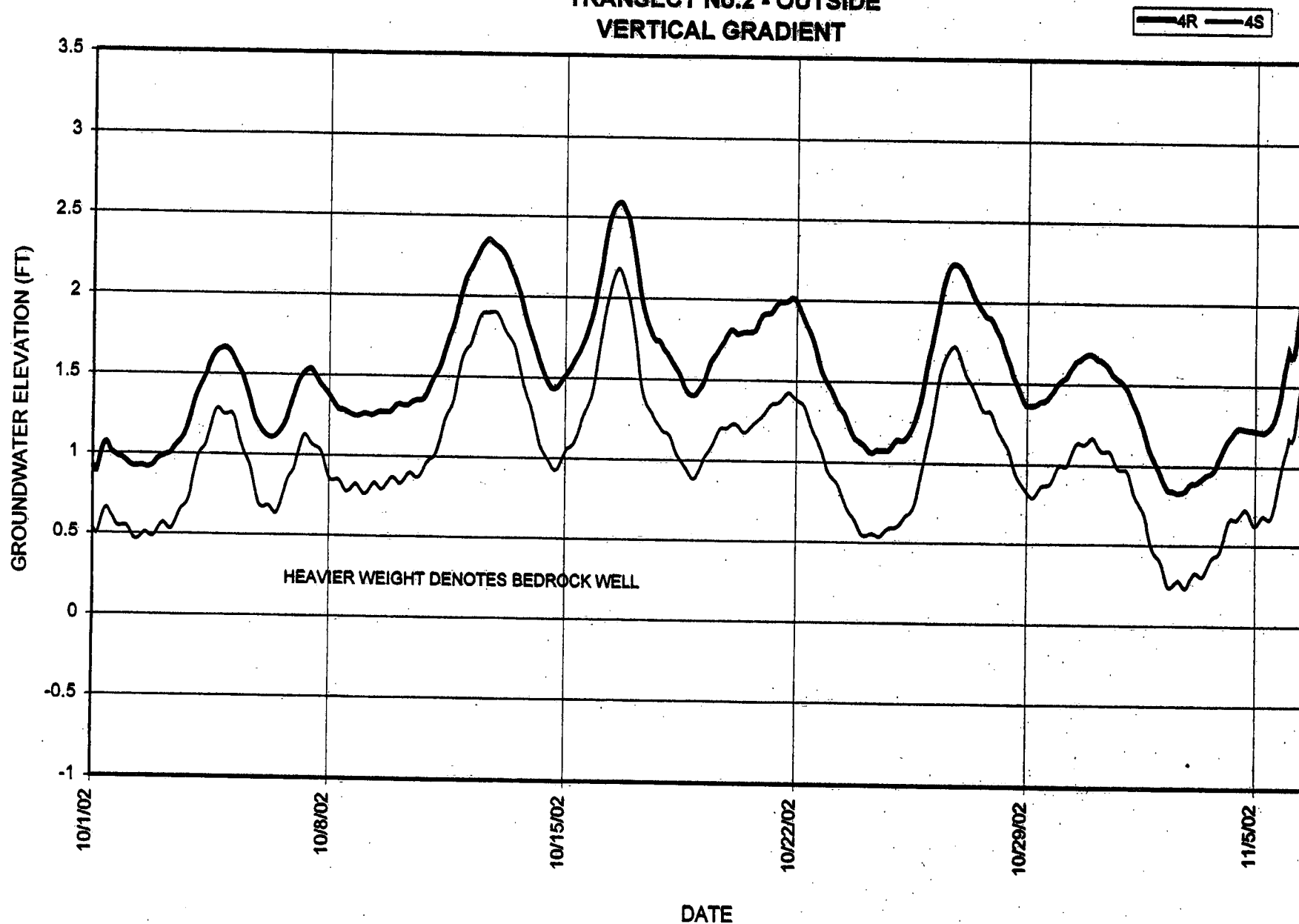
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #9
TRANSECT No.4 (OSA)
SAND & GRAVEL UNIT



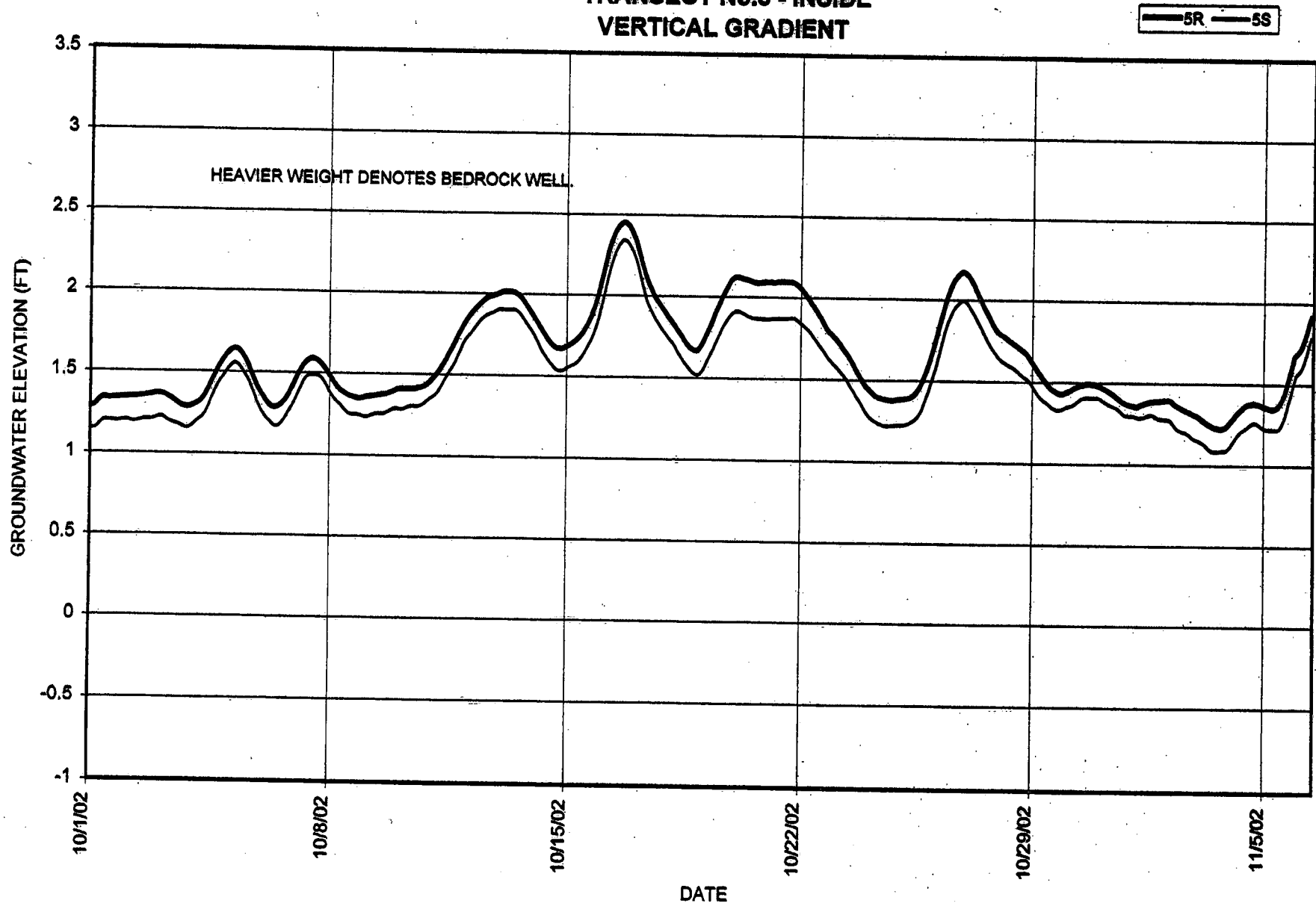
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #10
TRANSECT No.2 - INSIDE
VERTICAL GRADIENT



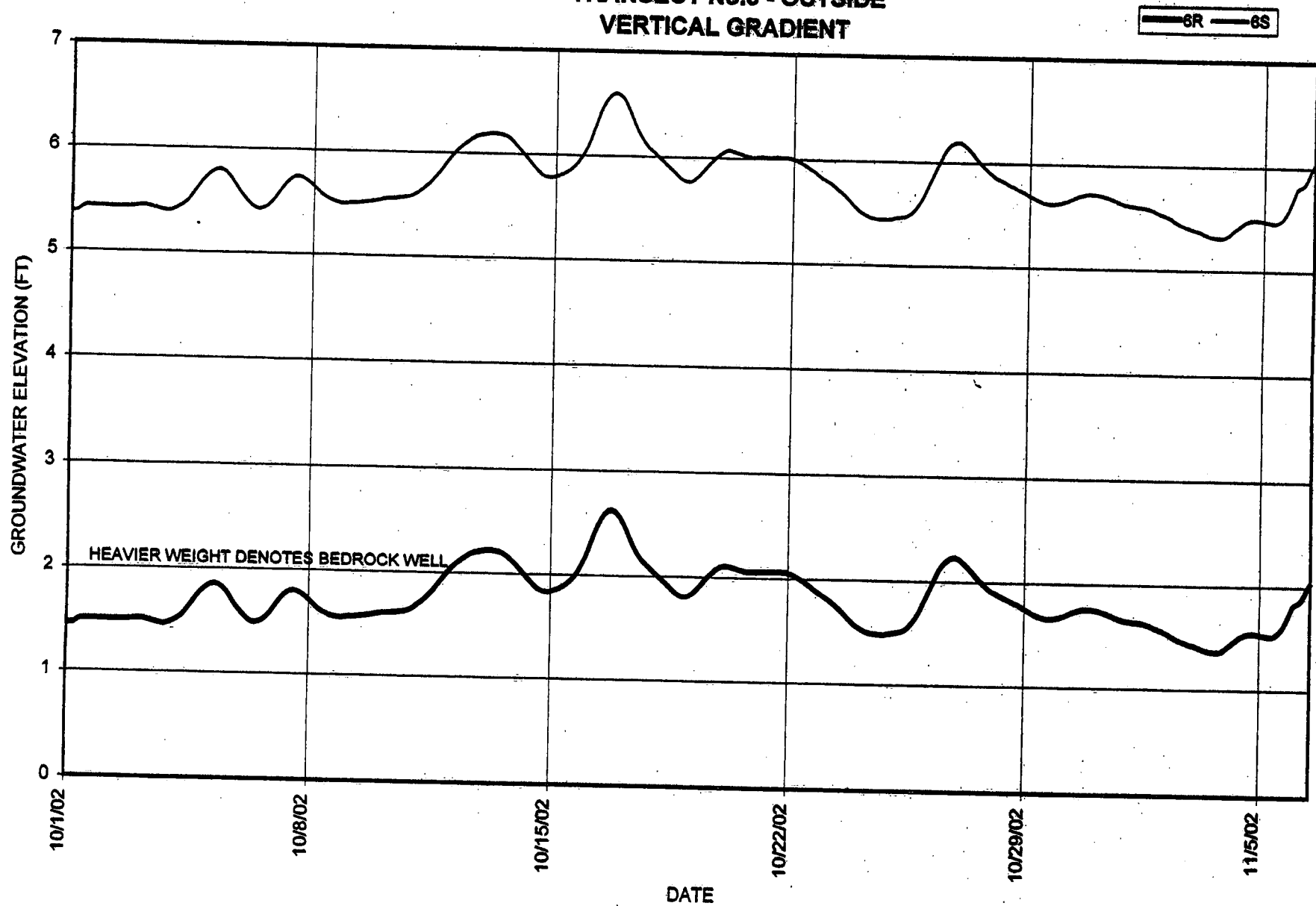
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #11
TRANSECT No.2 - OUTSIDE
VERTICAL GRADIENT



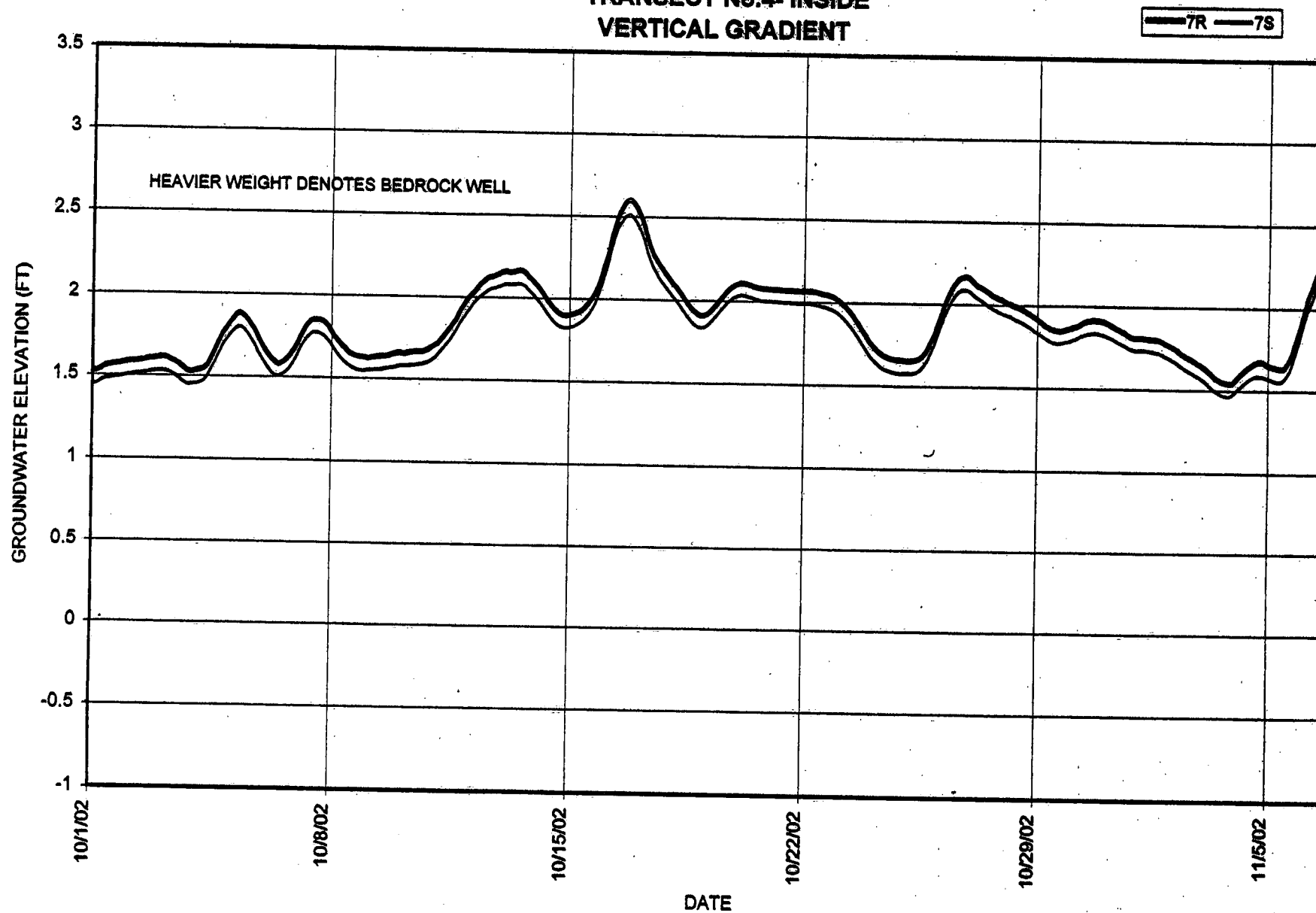
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #12
TRANSECT No.3 - INSIDE
VERTICAL GRADIENT



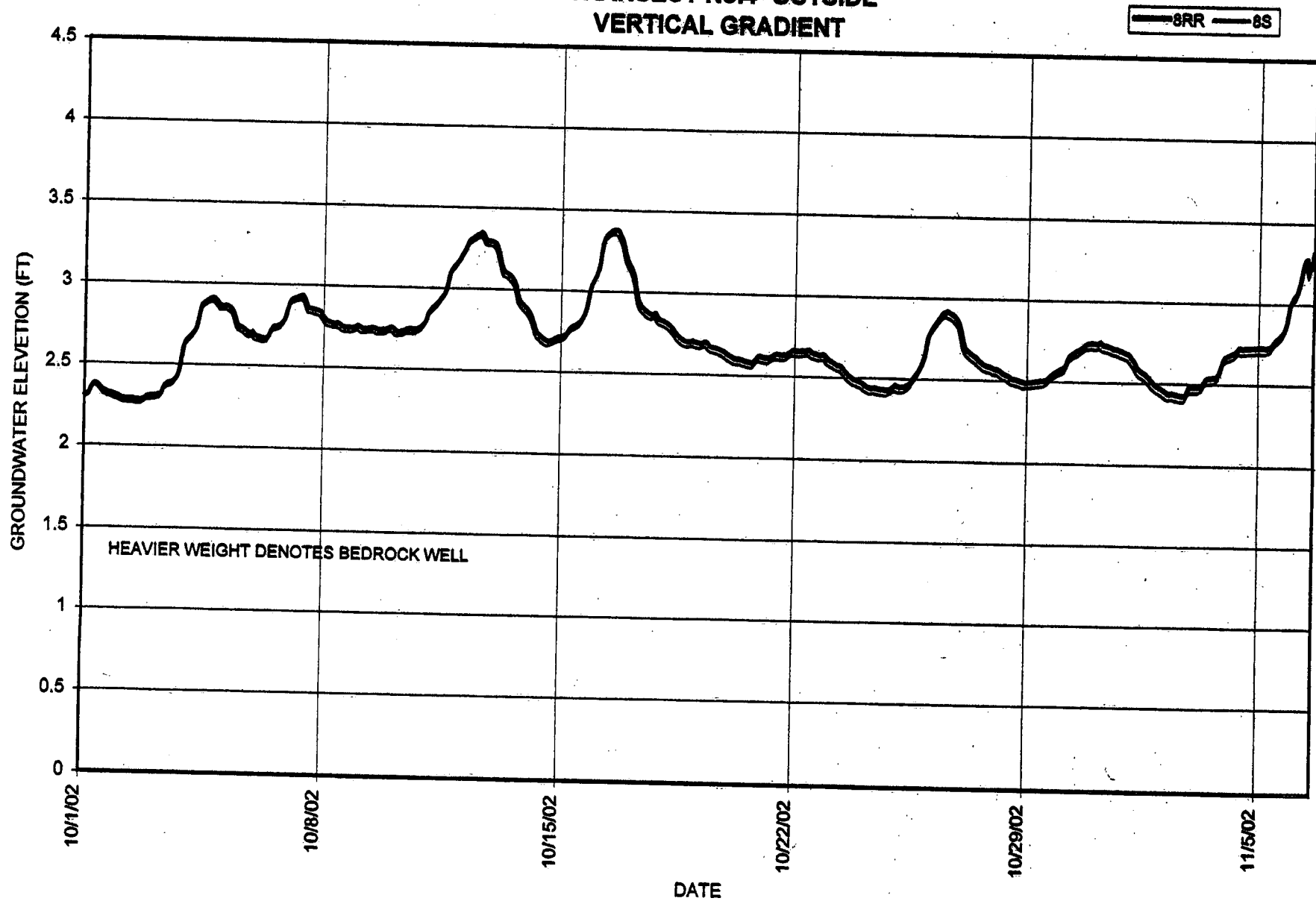
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #13
TRANSECT No.3 - OUTSIDE
VERTICAL GRADIENT



KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #14
TRANSECT No.4- INSIDE
VERTICAL GRADIENT



KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #15
TRANSECT No.4- OUTSIDE
VERTICAL GRADIENT



ATTACHMENT 2



IT Corporation

Crossroads Corporate Center
One International Boulevard, Suite 700
Mahwah, NJ 07495-0086
Tel. 201.512.5700
Fax. 201.512.5786

A Member of The IT Group

June 27, 2001
Project 796201

Carl Januszkiewicz
Waste Management, Inc
Kin-Buc Landfill Treatment Plant
383 Meadow Road
Edison, NJ 08817

Re: Evaluation of Head Levels at Transect 1

Dear Mr. Januszkiewicz:

We have completed an evaluation of the hydraulic characteristics at Transect 1 with specific focus on the lack of intragradient conditions associated with the high water levels in W-1G (inside of the slurry wall) relative to those levels in W-2G (outside of the wall).

While intragradient conditions were evident at the outset of the hydraulic monitoring program in April 1996, these conditions have generally not been maintained. Specifically, based on a review of historical hydrographs, intragradient conditions were evident initially from approximately April to July 1996, and April to June 1997. Thereafter, to more recent events, intragradient conditions have been observed intermittently and for shorter periods of time.

Attachment 1 presents a hydrograph at Transect 1 encompassing the period from September 1998 to December 2000. As seen on the hydrograph, there were periods of time when intragradient conditions were not being maintained.

As opposed to the other "G" series monitoring wells that are located in refuse, wells 1G and 2G at Transect 1 are actually located in a silt and clay deposit. Attachment 2 contains the boring logs for these 2 installations. In-situ hydraulic conductivity testing performed at Transect 1 indicated permeabilities of 10^{-7} cm/sec and 10^{-5} cm/sec in W-1G and W-2G, respectively. Accordingly, a source of recharge to the overburden soils in the area of W-1G would not readily drain away, and therefore, higher heads could result.

Well 1G sampling events (November 1998, October 1999, October 2000) can be seen on the hydrograph as sharp vertical drops in groundwater levels. Due to the low permeability of the surrounding materials, the groundwater levels required several months to recover. Since the final cover extends 10 feet past the slurry wall, the source of the groundwater that is recharging W-1G is unknown at present.

The hydraulic gradient between W-1G and W-1R is vertically downward which rules out the bedrock as being a source of groundwater recharge. Based on a recent visual inspection of the area around Transect 1, the cap appears to be good condition and there were no signs that the cap integrity has been compromised.

Figure 1 depicts the conceptual model of the hydraulic interrelationship across Transect 1 showing water level measurements that depict the lack of intragradient conditions across the

Carl Januszkiewicz
June 27, 2001
Page 2

Project 796201

slurry wall. The head levels in W-2G (outside the slurry wall) are generally at elevation 12 to 13 feet msl with periodic and short term increases to about 15 feet msl. The water level in the well sometimes falls below the level of the transducer. This is characterized by a flat straight line on the hydrographs as shown on Attachment 1. Head levels in W-1G (inside the slurry wall), on the other hand, are often greater with elevations as high as 15 to 16 feet msl being recorded.

It is evident from a review of Figure 1 that the drop in topography outside of the slurry wall toward Mill Brook, coupled with the higher permeability of W-2G relative to W-1G, would promote a more rapid decrease of head levels in the latter. This suggests that intragradient conditions may not be consistently attainable at this transect in any event. This notwithstanding however, and as depicted on Figure 1, it is important to note that the leachate collection system represents a hydraulic sink within the containment system. As such, groundwater in the vicinity of W-1G would drain toward the sink mitigating concerns of outward flow.

The leachate collection line runs parallel to the slurry wall and at its closest point is only about 20 feet away from Transect 1. Several cleanouts are located along the collection line with the closest, Cleanout 16, only about 65 feet from Transect 1. Leachate level measurements obtained from the cleanouts during December 2000 and June 2001 indicate a leachate level of 10 to 11 feet msl along the collection line as shown in Table 1. The leachate levels observed suggest that the leachate collection system is presently operating effectively.

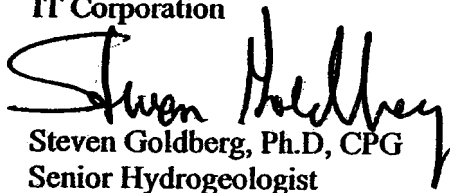
Recommendations

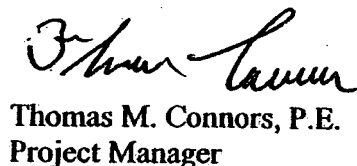
Based on the above, it is recommended that during subsequent monitoring events at the site, measurements of leachate levels in Cleanouts 14 through 16 be recorded to verify that the leachate collection system is operating effectively. If liquid levels in the cleanouts increase above 12 to 13 feet msl, then maintenance of the collection line is recommended. Subsequent reports to EPA should include a discussion of the leachate collection system and its role as serving as a hydraulic sink within the containment system.

We trust you find this information useful. If you have any questions, please do not hesitate to contact us.

Sincerely,

IT Corporation


Steven Goldberg, Ph.D, CPG
Senior Hydrogeologist


Thomas M. Connors, P.E.
Project Manager

Attachments

IT
IT CORPORATION

WASTE MANAGEMENT INC.

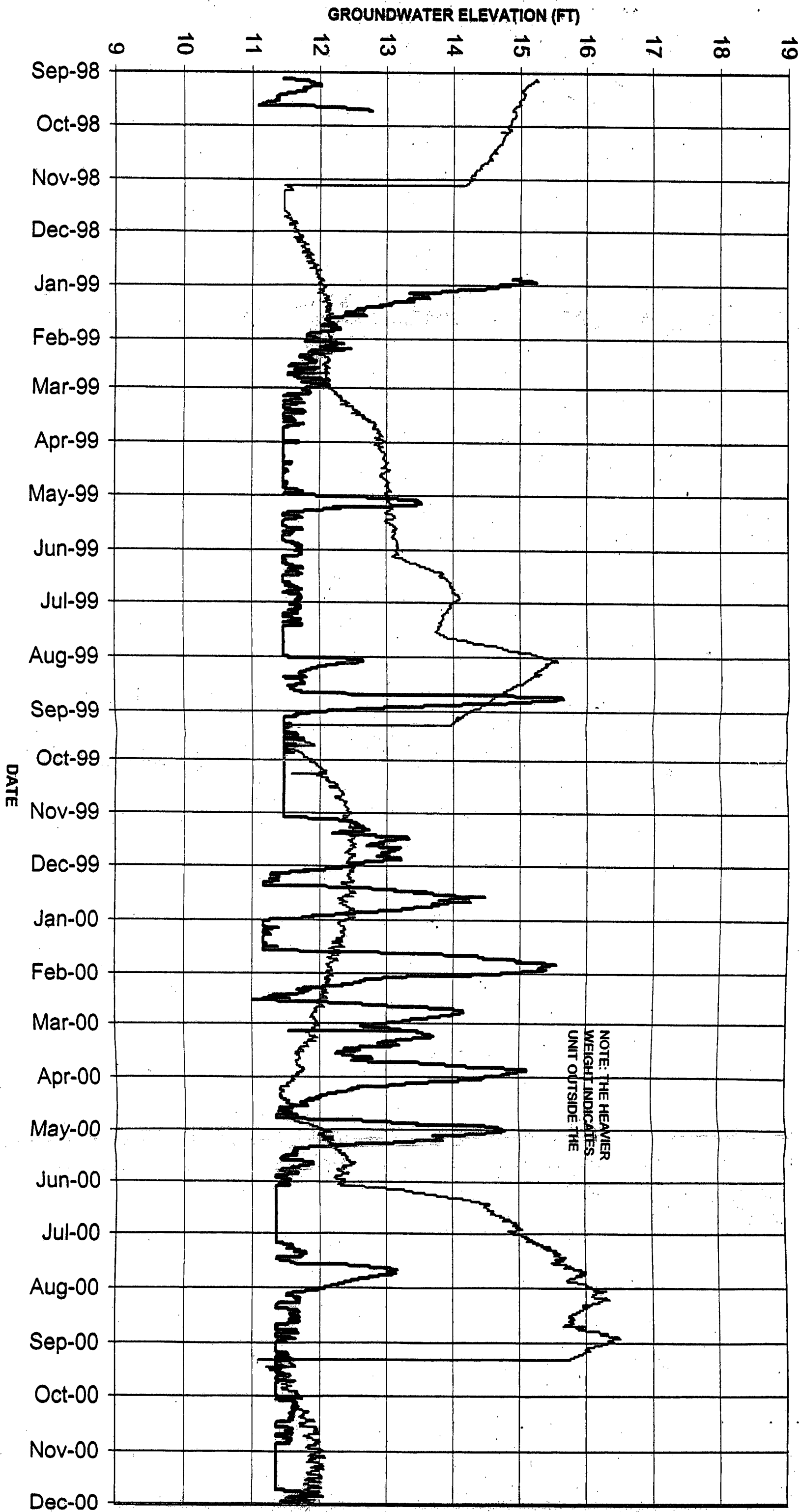
FIGURE 1
CROSS SECTION
TRANSECT 1
KIN-BUC LANDFILL
EDISON, NEW JERSEY

Leachate Cleanout Monitoring 2001

[illegible]

ATTACHMENT 1

KIN-BUC LF GROUNDWATER ELEVATION HYDROGRAPH AT TRANSECT NO.1 REFUSE UNIT



ATTACHMENT 2

MONITORING WELL RECORD

Well Permit No. 25 - 46506
 Atlas Sheet Coordinates 25 : 45 : 428

OWNER IDENTIFICATION - Owner KIN-HUC INC.
 Address 200 CENTINIAL AVE.
 City PISCATAWAY State NJ Zip Code _____

WELL LOCATION - If not the same as owner please give address. Owner's Well No. 2G
 County MIDDLESEX Municipality EDISON TWP Lot No. 420 Block No. 3C
 Address 383 Meadows Road, Edison, NJ

TYPE OF WELL (as per Well Permit Categories) MONITORING Date well completed 2 / 15 / 95
 Regulatory Program Requiring Well CERCLA Case I.D. # NJD049860836
 CONSULTING FIRM/FIELD SUPERVISOR (If applicable) _____ Tele. # _____

WELL CONSTRUCTION

Total depth drilled 15.6 ft.

Well finished to 15 ft.

Borehole diameter:

Top 8 in.

Bottom 8 in.

Well was finished: ☒ above grade

☐ flush mounted

Finished above grade, casing
 height (stick up) above land
 surface 4 ft.

Was steel protective casing installed?

☐ Yes ☒ No

Static water level after drilling _____ ft.

Water level was measured using _____

Well was developed for N/A hours at N/A gpm

Method of development N/A

Was permanent pumping equipment installed? ☐ Yes ☒ No

Pump capacity N/A gpm

Pump type: N/A

Drilling Method HSA

Drilling Fluid _____ Type of Rig B-61

Name of Driller Chad Chism

Health and Safety Plan submitted? ☐ Yes ☒ No

Level of Protection used on site (circle one) None D C (B) A

J. License No. 0013753-001375

Name of Drilling Company HARDIN-HUBER, INC.

	Depth to Top (ft.) [From land surface]	Depth to Bottom (ft.)	Diameter (inches)	Type and Material
Inner Casing	+4	5	2	Sch 40 PVC
Outer Casing (Not Protective Casing)				
Screen (Note slot size)	5	15	2	Sch 40 PVC .010
Tail Piece				
Gravel Pack	3	15.6	8	#00 Ricci
Annular Seal/Grout	0	3	8	Bentonite slurry
Method of Grouting	tremie			

GEOLOGIC LOG (Copies of other geologic logs and/or geophysical logs should be attached.)

0 - 15.6 red dry stiff clay,
 some silt

I certify that I have drilled the above-referenced well in accordance with all well permit requirements and all applicable state rules and regulations.

Driller's Signature Chad Chism Date 2/15/95

COPIES: White - DEP Canary - Driller Pink - Owner Goldenrod - Health Dept.

MONITORING WELL RECORD

Well Permit No. 25 - 46505
Atlas Sheet Coordinates 25 : 45 : 428

OWNER IDENTIFICATION - Owner KIN-HIC, INC.
Address 200 CENTENIAL AVE.
City PISCATAWAY State NJ Zip Code _____

WELL LOCATION - If not the same as owner please give address. Owner's Well No. 1G
County MIDDLESEX Municipality EDISON TWP Lot No. 400 Block No. 3C
Address 383 Meadows Road, Edison, NJ

TYPE OF WELL (as per Well Permit Categories) MONITORING Date well completed 2 / 15 / 95
Regulatory Program Requiring Well CERCLA Case I.D. # NJD049860836

CONSULTING FIRM/FIELD SUPERVISOR (if applicable) _____ Tele. # _____

WELL CONSTRUCTION

Total depth drilled 15.6 ft.

Well finished to 15 ft.

Borehole diameter:

Top 8 in.

Bottom 8 in.

Well was finished: ☒ above grade
☐ flush mounted

If finished above grade, casing
height (stick up) above land
surface _____ ft.

	Depth to Top (ft.) [From land surface]	Depth to Bottom (ft.)	Diameter (inches)	Type and Material
Inner Casing	+4	5	2	Sch 40 PVC
Outer Casing (Not Protective Casing)				
Screen (Note slot size)	5	15	2	Sch 40 PVC .020
Tail Piece				
Gravel Pack	3	15.6	8	#2 Ricci
Annular Seal/Grout	0	5	8	Bentonite slurry
Method of Grouting	tremie			

Was steel protective casing installed?
☐ Yes ☒ No

Static water level after drilling _____ ft.

Water level was measured using _____

Well was developed for N/A hours at N/A gpm

Method of development N/A

Was permanent pumping equipment installed? ☐ Yes ☒ No

Pump capacity N/A gpm

Pump type: N/A

Drilling Method HSA

Drilling Fluid _____ Type of Rig B-61

Name of Driller Chad Chism

Health and Safety Plan submitted? ☐ Yes ☒ No

Level of Protection used on site (circle one) None D C B A

N.J. License No. 0013753-001375

Name of Drilling Company HARDIN-HUBER, INC.

GEOLOGIC LOG (Copies of other geologic logs and/or geophysical logs should be attached.)

0 - 15.6 red gray dry stiff
clay, some silt

I certify that I have drilled the above-referenced well in accordance with all well permit requirements and all applicable State rules and regulations.

Driller's Signature Chad Chism Date 2/15/95

COPIES: White - DEP Canary - Driller Pink - Owner Goldenrod - Health Dept.

February 3, 2003
Project 791186

Mr. Carl Januszkiewicz
Waste Management, Inc.
Kin-Buc Landfill Treatment Plant
383 Meadow Road
Edison, NJ 08817

Re: Hydraulic Monitoring for November and December 2002

Dear Mr. Januszkiewicz:

Site visits were completed on December 6, 2002 and January 2, 2003 to download water level recorder data and obtain manual water level measurements. The following is an update of the hydraulic monitoring for the months of November 2002 and December 2002 at the Kin-Buc Landfill. This information is to be included in the quarterly report, which is to be submitted to the EPA by mid-February 2003.

The minimum, maximum, and average water elevations recorded at each well are included in Table 1. Table 2 shows the troll water elevations versus the manual water elevations. The continuous water level elevation data when compared with manual readings indicated that the miniTrolls are functioning properly and are recording accurate data with the exception of Well 13G.

During the site visit on December 6, 2002, the replacement miniTroll (serial number 10275), sent by In-Situ, Inc. was installed in Well 15S. In addition, an SP 4000 Troll was temporarily installed in Well 13G.

In-Situ Inc. repaired and returned the miniTroll that had previously malfunctioned in Well 13G (serial number 6171). The battery and pressure sensor were replaced. The miniTroll will be installed during the next site visit in early February.

Hydrographs have been prepared for each of the transect locations and are enclosed for your reference as Attachment No. 1.

The water levels in wells on the outside of the slurry wall vary over the course of the day due to the tidal influence at the site. For clarity, Hydrograph Nos. 6 through 15 show the average water level in the well over a 24-hour period (12 hours before, and 12 hours after).

Refuse

As defined in the Record of Decision (ROD) for OU-1, the performance objective for the refuse unit calls for the pumping of leachate to establish inward gradients across the slurry wall with the additional benefit of reducing downward flow into the underlying sand and gravel unit. Based on the hydrographs the following is presented.

Transect 1-Refuse (1G/2G)/Hydrograph No. 1 - Intragradients conditions were not observed during the month of November, but were maintained during the month of December. The average monthly water elevation for November at Well 1G (inside) and Well 2G (outside) was 11.73 and 12.10 feet msl, respectively. The average monthly water elevation for December at Well 1G (inside) and Well 2G (outside) was 11.34 and 12.59 feet msl, respectively. Water level elevation measurements taken from Leachate Collection Cleanouts Nos. 14 through 16 are included in Table 3, and indicate that the leachate collection system is functioning properly. The fact that the leachate collection system is functioning properly suggests significant capture of leachate. The evaluation of the hydraulic conditions in the refuse at Transect 1 is provided in Attachment No.2.

Transect 2-Refuse (3G/4G)/Hydrograph No. 2 - Intragradients conditions were maintained throughout the months of November and December. The average monthly water elevation for the month of November at Well 3G (inside) and Well 4G (outside) was 9.94 and 11.18 feet msl, respectively. The average monthly water elevation for the month of December at Well 3G (inside) and Well 4G (outside) was 9.89 and 11.23 feet msl, respectively.

Transect 3-Refuse (5G/6G)/Hydrograph No. 3 - Intragradients conditions were maintained throughout the months of November and December. The average monthly water elevation for the month of November at Well 5G (inside) and Well 6G (outside) was 4.39 and 13.33 feet msl, respectively. The average monthly water elevation for the month of December at Well 5G (inside) and Well 6G (outside) was 4.27 and 13.37 feet msl, respectively.

Transect 4-Refuse Oil Seeps Area (13G/15G)/Hydrograph No. 4 - No continuous water level data for Well 13G is available for the month November due to the previously noted (October Hydraulic Monitoring Letter) memory chip malfunction in the miniTroll (serial number 6171). The SP 4000 Troll was installed in Well 13G on December 6, 2002. Therefore, data is available from December 6, 2002 forward. Intragradients conditions were maintained throughout the month of December. The average monthly water elevation for the month of November at Well 15G (inside) and Well 13G (outside) was 1.05 and 6.62 (taken from manual water level readings) feet msl, respectively. The average monthly water elevation for the month of December at Well 15G (inside) and Well 13G (outside) was 1.09 and 2.97 feet msl, respectively.

Transect 5-Refuse (9G/10G)/Hydrograph No. 5 – Intragradient conditions were not maintained throughout the months of November and December. It should be noted that there were two spikes at different time periods noted on the hydrograph. The first spike (November 13) is at the time when the groundwater sampling took place and the second (December 6) is when the miniTroll was removed to check for any discoloration during the EMCON/OWT, Inc. routine site visit. The average monthly water elevation for the month of November at Well 9G (inside) and Well 10G (outside) was 7.64 and 7.22 feet msl, respectively. The average monthly water elevation for the month of December at Well 9G (inside) and Well 10G (outside) was 7.34 and 7.63 feet msl, respectively.

Sand and Gravel/Bedrock

For the sand and gravel unit, the performance objectives call for pumping of sand and gravel groundwater to prevent flow toward the slurry wall and to impose upward hydraulic gradients from the bedrock to the sand and gravel. An additional benefit would be the establishment of inward gradients across the slurry wall within the sand and gravel unit. The following is a description of the flow characteristics based on visual observation of the hydrographs

Horizontal Flow

Transect 2-Sand and Gravel (3S/4S)/Hydrograph No. 6 – Intragradient conditions were not consistently maintained for the months of November and December. The average monthly water elevations for the month of November at Well 3S (inside) and Well 4S (outside) was 0.68 and 0.76 feet msl, respectively. The average monthly water elevations for the month of December at Well 3S (inside) and Well 4S (outside) was 0.38 and 0.62 feet msl, respectively.

Transect 3-Sand and Gravel (5S/6S)/Hydrograph No. 7 – Intragradient conditions were maintained throughout the months of November and December. The average water elevation for the month of November for Well 5S (inside) and Well 6S (outside) was 1.47 and 5.55 feet msl, respectively. The average water elevation for the month of December for Well 5S (inside) and Well 6S (outside) was 1.27 and 5.45 feet msl, respectively.

Transect 4-Sand and Gravel (7S/8S)/Hydrograph No. 8- Intragradient conditions were maintained throughout the months of November and December. The average monthly water elevation for the month of November at Well 7S (inside) and Well 8S (outside) was 1.76 and 2.44 feet msl, respectively. The average monthly water elevation for the month of December at Well 7S (inside) and Well 8S (outside) was 1.53 and 2.41 feet msl, respectively.

Transect 4 Sand and Gravel Oil Seeps Area (13S/15S)/Hydrograph No. 9 – Intragradient conditions were not evident during the months of November and December.

The average monthly water elevation for the month of November at Well 15S (inside) and Well 13S (outside) was 5.85 and 2.22 feet msl, respectively. The average monthly water elevation for the month of December at Well 15S (inside) and Well 13S (outside) was 5.69 and 2.17 feet msl, respectively. Water levels from Well 15G in the refuse unit are included on the hydrograph for comparison.

Vertical Flow-Inside Slurry Wall

Transect 2-Vertical Gradient (3S/3RR)-Inside/Hydrograph No.10 – Upward gradient conditions were not observed between the bedrock and overlying sand & gravel units inside the slurry wall for the months of November and December. The average monthly water elevation for the month of November at Well 3S (sand & gravel) and Well 3RR (bedrock) was 0.68 and 0.56 feet msl, respectively. The average monthly water elevation for the month of December at Well 3S (sand & gravel) and Well 3RR (bedrock) was 0.38 and 0.29 feet msl, respectively.

Transect 3-Vertical Gradient (5R/5S)-Inside/Hydrograph No.12 – Upward gradient conditions were not observed throughout the months of November and December between the bedrock and overlying sand & gravel units inside the slurry wall. The average monthly water elevation for the month of November at Well 5S (sand & gravel) and Well 5R (bedrock) was 1.47 and 1.49 feet msl, respectively. The average monthly water elevation for the month of December at Well 5S (sand & gravel) and Well 5R (bedrock) was 1.27 and 1.34 feet msl, respectively. Please note that the test stopped running in Well 5R on December 28, 2002 at 12:00. A new test was started on January 2, 2003 at 11:00.

Transect 4-Vertical Gradient (7R/7S)-Inside/Hydrograph No.14 – Slight upward gradient conditions were observed between the bedrock and overlying sand & gravel units inside the slurry wall throughout the month of December, but were not observed throughout the month of November. The average monthly water elevation for the month of November at Well 7S (sand & gravel) and Well 7R (bedrock) was 1.76 and 1.75 feet msl, respectively. The average monthly water elevation for the month of December at Well 7S (sand & gravel) and Well 7R (bedrock) was 1.53 and 1.60 feet msl, respectively. The difference in average monthly water elevations was less than 0.1 feet.

Vertical Flow-Outside Slurry Wall

Transect 2-Vertical Gradient (4S/4R)-Outside/Hydrograph No.11 – Upward gradient conditions were observed between the bedrock and overlying sand & gravel units outside the slurry wall for the months of November and December. The average monthly water elevation for the month of November at Well 4S (sand & gravel) and Well 4R (bedrock) was 0.76 and 1.41 feet msl, respectively. The

average monthly water elevation for the month of December at Well 4S (sand & gravel) and Well 4R (bedrock) was 0.62 and 1.17 feet msl, respectively.

Transect 3-Vertical Gradient (6R/6S)-Outside/Hydrograph No. 13 – Upward gradient conditions were not observed between the bedrock and overlying sand & gravel units outside the slurry wall for the months of November and December. The average monthly water elevation for the month of November at Well 6S (sand & gravel) and Well 6R (bedrock) was 5.55 and 1.72 feet msl, respectively. The average monthly water elevation for the month of December at Well 6S (sand & gravel) and Well 6R (bedrock) was 5.45 and 1.71 feet msl, respectively.

Transect 4-Vertical Gradient (8RR/8S)-Outside/Hydrograph No. 15 – Slight upward gradient conditions were observed between the bedrock and overlying sand & gravel units outside the slurry wall throughout the months of November and December. The average monthly water elevation for the month of November at both Well 8S (sand & gravel) and Well 8RR (bedrock) was 2.44 and 2.45 feet msl, respectively. The average monthly water elevation for the month of December at both Well 8S (sand & gravel) and Well 8RR (bedrock) was 2.41 and 2.48 feet msl, respectively. The difference in average monthly water elevations for November and December were 0.01 and 0.07 feet, respectively.

An initial review of the hydrographs indicates that certain of the performance objectives associated with the sand and gravel and bedrock units may not be met, specifically associated with the uniform achievement of upward gradients from the bedrock to the overlying sand and gravel (e.g. Hydrographs 10 and 13), and inward gradients across the slurry wall within the sand and gravel (Hydrograph 6). However previous investigations performed at the site would indicate that complete control of OU-1 groundwater can be achieved notwithstanding indications of downward flow from the sand and gravel to the bedrock, or outward flow across the slurry wall within the sand and gravel unit. This is based on the findings of the considerable pumping influence of the sand and gravel pumping wells, in particular S&G#2, in achieving hydraulic control at the site (see Groundwater Pumping Well Performance Evaluation Report, July 2000).

The influence of the pumping well can be demonstrated by a review of plan view groundwater contour map of the sand and gravel (Figure 1) and equipotential profiles and vector diagrams (Figures 2, 3, and 4) that have been prepared for a period of time when the vertical gradient between the sand and gravel and the bedrock was downward, and flow across the slurry wall within the sand and gravel unit was outward. For this evaluation, a snapshot of groundwater elevations from the monitoring wells and pumping wells was obtained for November 23, 2002 and December 23, 2002. At this time, S&G#2 was pumping at a rate of about 12 gallons per minute (gpm) on November 23 and about 14.5 gallons per minute (gpm) on December 23, while S&G#3 was pumping at a rate of

3 gpm for both periods. This resulted in a total of approximately 15 gpm or about 20,800 gallons per day on November 23, and approximately 17.5 gpm or about 24,000 gallons per day on December 23. There was a downward vertical gradient observed between the sand and gravel and the bedrock inside the slurry wall at Transect No.2 in November and outside the slurry wall at Transect 3 in November and December as evidenced by higher heads in the sand and gravel wells relative to bedrock wells. There was also a higher head within the sand and gravel inside the slurry wall relative to the sand and gravel outside the slurry wall at Transect No. 2 in November.

Figures 1-4 incorporate the heads induced by pumping and show the considerable pumping influence of S&G#2. Specifically, groundwater flowing downward from the sand and gravel into the bedrock is subsequently induced toward the pumping well. This occurs both inside and outside of the slurry wall. Also, groundwater within the sand and gravel unit is induced toward the pumping well. The considerable pumping influence demonstrated at S&G#2, in conjunction with the fact that natural groundwater gradients in both the sand & gravel and bedrock flow predominantly towards the area of S&G#2, result in the complete capture of OU-1 groundwater at these pumping rates.

Groundwater and Leachate Collection

Based on data provided by U.S. Filter, the following volumes of groundwater and leachate were extracted from the sand & gravel wells and leachate collection system for the period from November 1 to November 30, 2002:

S&G No. 1 Groundwater	S&G No. 2 Groundwater	S&G No. 3 Groundwater	S&G No. 4 Groundwater	Leachate
0 gal.	485,989 gal.	121,533 gal.	23,784 gal.	47,508 gal.
0 gpd	16,758 gpd	4,051 gpd	793 gpd	1,584 gpd

and for the period from December 1 to December 31, 2002:

S&G No. 1 Groundwater	S&G No. 2 Groundwater	S&G No. 3 Groundwater	S&G No. 4 Groundwater	Leachate
12,400 gal.	610,296 gal.	120,629 gal.	0 gal.	63,589 gal.
400 gpd	19,687 gpd	3,891 gpd	0 gpd	2,051 gpd

For the month of November, a total of 631,306 gallons of groundwater was collected. The average daily groundwater extraction rate for all of the wells was 21,044 gpd. The extraction rate from S&G No. 2 was 16,758 gpd, from S&G No. 3 it was 4,051 gpd, and the extraction rate from S&G No. 4 was 793 gpd. The leachate extraction rate of 1,584 gpd met the recommended rate of 1,500 gpd.

For the month of December, a total of 743,325 gallons of groundwater was collected. The average daily groundwater extraction rate for all of the wells was 23,978 gpd. The extraction rate from S&G No. 1 was 400 gpd, from S&G No. 2 it was 19,687 gpd, and the extraction rate from S&G No. 3 was 3,891 gpd. The leachate extraction rate of 2,051 gpd exceeded the recommended rate of 1,500 gpd.

CONCLUSIONS

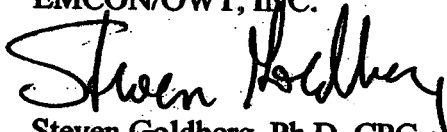
- Intragradiant conditions were maintained in the refuse unit at Transects 2, 3, and 4. With regards to Transect 5, head levels in W-10G (outside the wall) decreased from about 8.5 msl to 5.5 msl on about November 13. This may have been the result of sampling the well. There was another event on December 6 associated with removal of the troll. Between these two events, head levels were higher inside the wall. However, the leachate level in W-10G recovered sufficiently around December 10 to reestablish intragradiant conditions at this transect location.
- Intragradiant conditions were not indicated during the first part of December by the monitoring wells at Transect 1, although levels in the leachate collection system indicate intragradiant conditions are present at this location.
- Hydraulic control was maintained within OU-1 based on the analysis of the significant influence of S&G#2 in acting as a hydraulic sink for sand and gravel and bedrock groundwater. Groundwater flow in the sand and gravel and bedrock is ultimately captured by the pumping well resulting in overall containment of groundwater in OU-1.
- In view of the analysis presented herein, it is recommended that the combined groundwater pumping rates in the sand and gravel be maintained at 15,000 gpd with S&G#2 and S&G#3 pumping at 10,000 gpd and 5,000 gpd, respectively. These lower pumping rates will be evaluated to confirm continued hydraulic control of OU-1 groundwater.

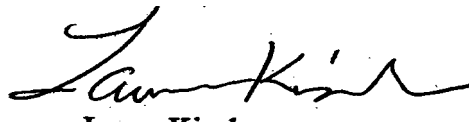
Mr. Carl Januszkiewicz
February 3, 2003
Page 8

Project 791186

We trust you find this information useful. If you have any questions, please do not hesitate to contact us.

EMCON/OWT, INC.


Steven Goldberg, Ph.D, CPG
Senior Hydrogeologist


Laura Kisala
Environmental Scientist

Attachments

cc: Glenn Grieb, US Filter



LEGEND:

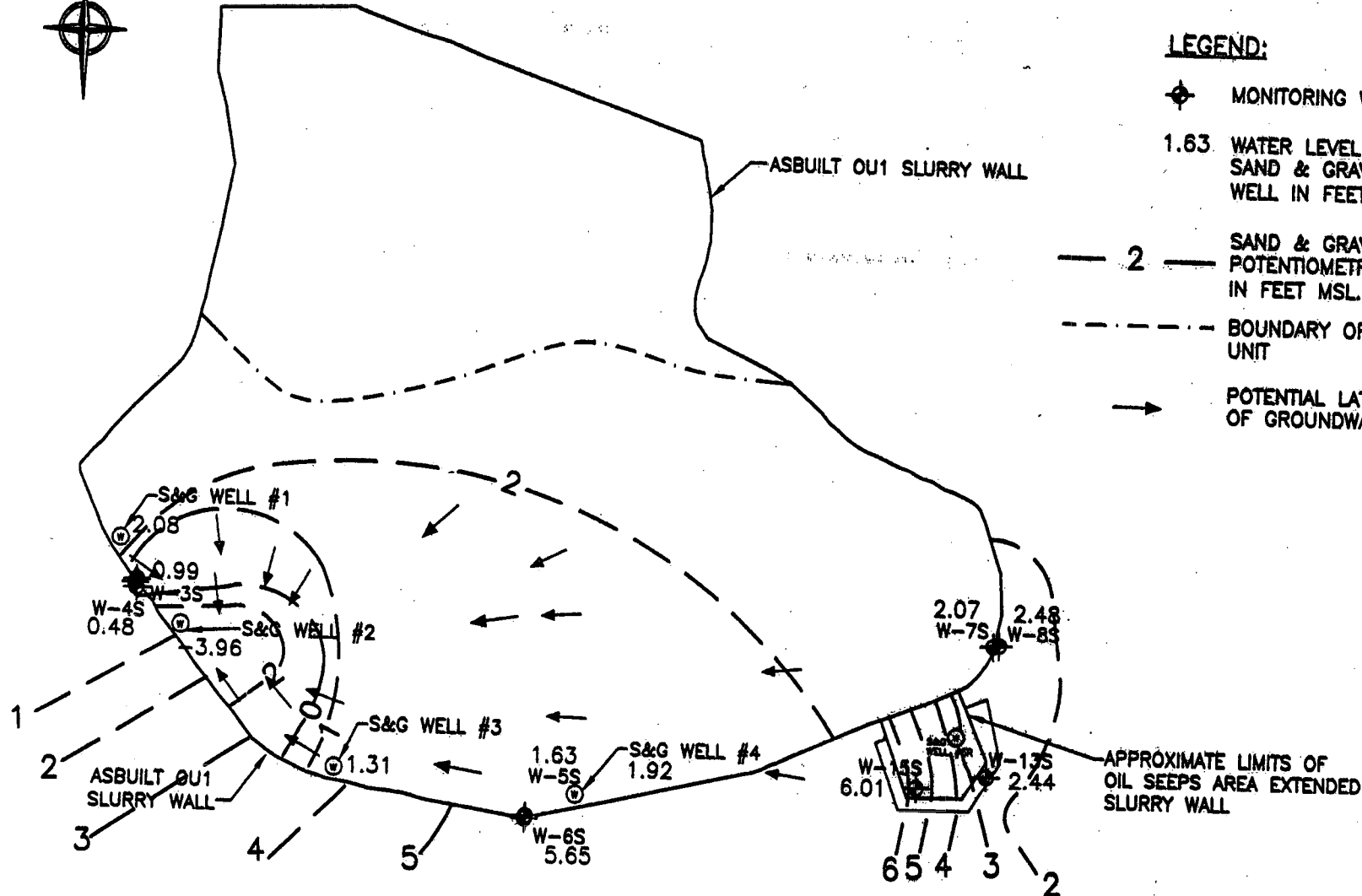
⊕ MONITORING WELL

1.63 WATER LEVEL ELEVATION IN SAND & GRAVEL MONITORING WELL IN FEET MSL, 11/23/02

— 2 — SAND & GRAVEL POTENTIOMETRIC SURFACE IN FEET MSL, 11/23/02

- - - - - BOUNDARY OF SAND & GRAVEL UNIT

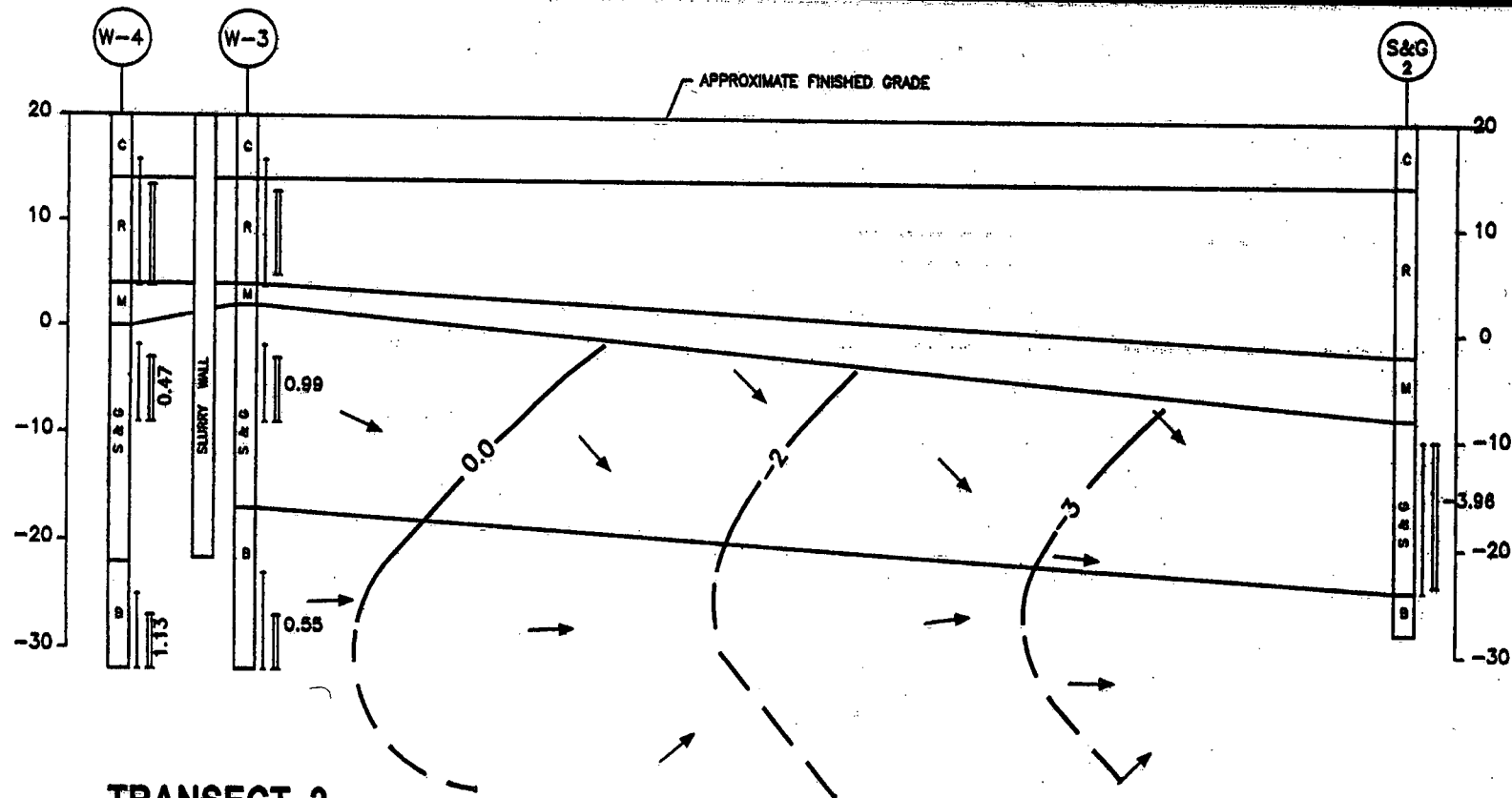
→ POTENTIAL LATERAL DIRECTION OF GROUNDWATER FLOW



0 400 800
 SCALE IN FEET

DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO. 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
FIGURE 1
 SAND & GRAVEL UNIT
 POTENTIOMETRIC SURFACE



TRANSECT 2

SCALE: N.T.S.

LEGEND:

B - BEDROCK
 C - CAP
 R - REFUSE
 M - MEADOW MAT
 S & G - SAND & GRAVEL

0.13

ELEVATION OF WATER LEVEL IN MONITORING WELL, IN FEET MSL, 11/23/02

0.5

GROUNDWATER EQUIPOTENTIAL IN BEDROCK AND SAND & GRAVEL, IN FEET MSL, 11/23/02

→

POTENTIAL VERTICAL DIRECTION OF GROUNDWATER FLOW.

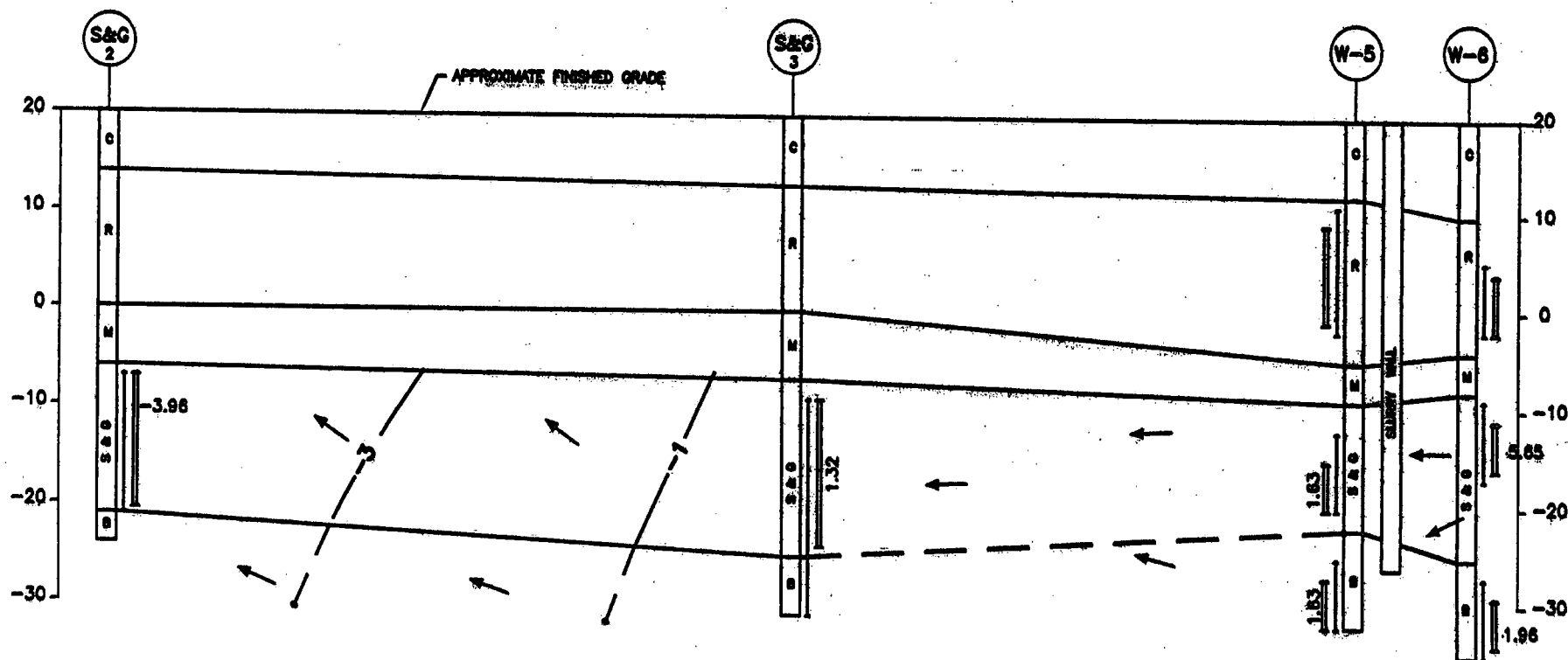
SAND PACK INTERVAL
 SCREENED INTERVAL



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO.
 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY

FIGURE 2
 TRANSECT 2
 HYDROGEOLOGIC CROSS SECTION ANALYSIS



TRANSECT 3

SCALE: N.T.S.

LEGEND:

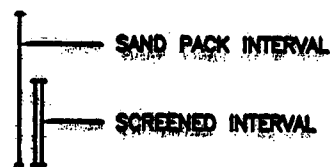
- B - BEDROCK
- C - CAP
- R - REFUSE
- M - MEADOW MAT
- S & G - SAND & GRAVEL

1.32

ELEVATION OF WATER LEVEL IN MONITORING WELL, IN FEET MSL, 11/23/02

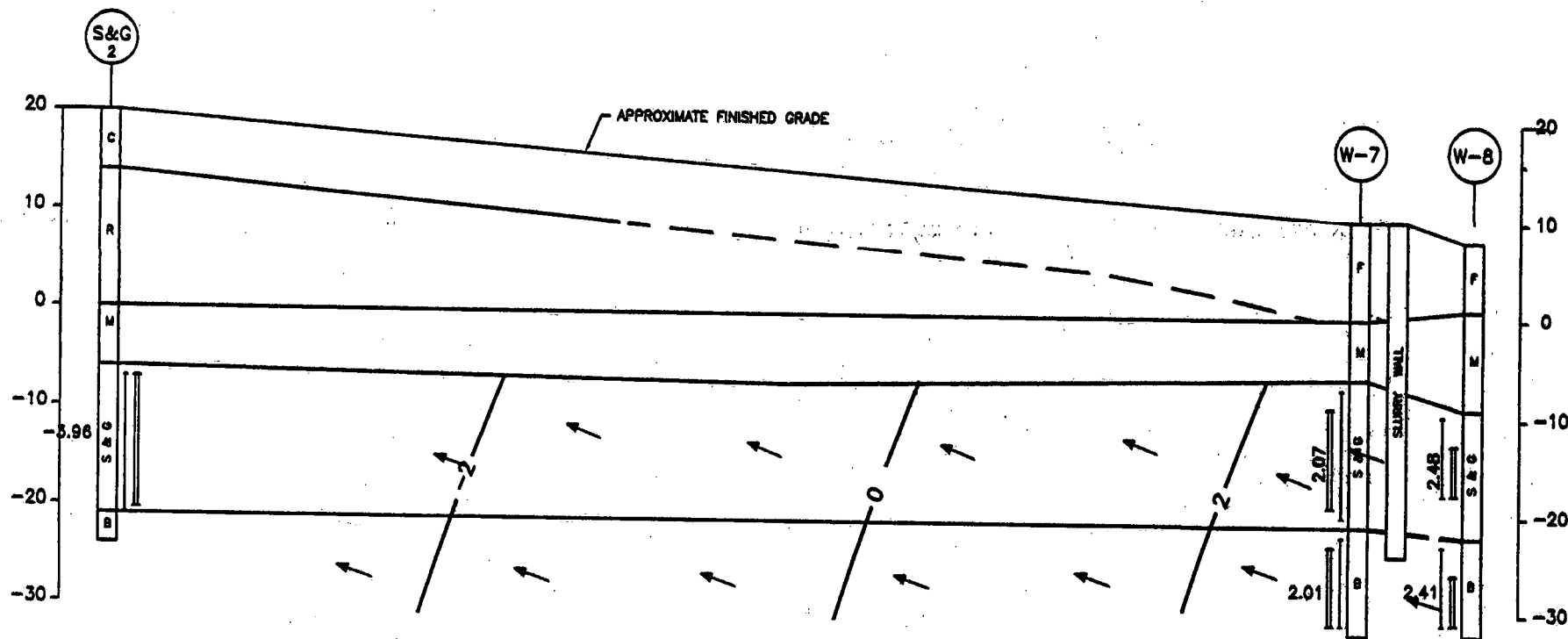
GROUNDWATER EQUIPOTENTIAL IN BEDROCK AND SAND & GRAVEL, IN FEET MSL, 11/23/02

POTENTIAL VERTICAL DIRECTION OF GROUNDWATER FLOW.



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO. 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
 FIGURE 3
 TRANSECT 3
 HYDROGEOLOGIC CROSS SECTION ANALYSIS

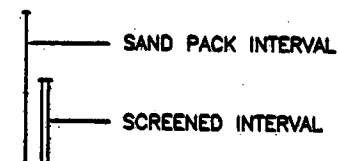


TRANSECT 4

SCALE: N.T.S.

LEGEND:

- | | | | |
|-------|-----------------|------|---|
| B | - BEDROCK | 2.31 | ELEVATION OF WATER LEVEL IN MONITORING WELL, IN FEET MSL, 11/23/02 |
| C | - CAP | -2- | GROUNDWATER EQUIPOTENTIAL IN BEDROCK AND SAND & GRAVEL, IN FEET MSL, 11/23/02 |
| R | - REFUSE | → | POTENTIAL VERTICAL DIRECTION OF GROUNDWATER FLOW. |
| M | - MEADOW MAT | | |
| S & G | - SAND & GRAVEL | | |



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO. 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
 FIGURE 4
 TRANSECT 4
 HYDROGEOLOGIC CROSS SECTION ANALYSIS



LEGEND:

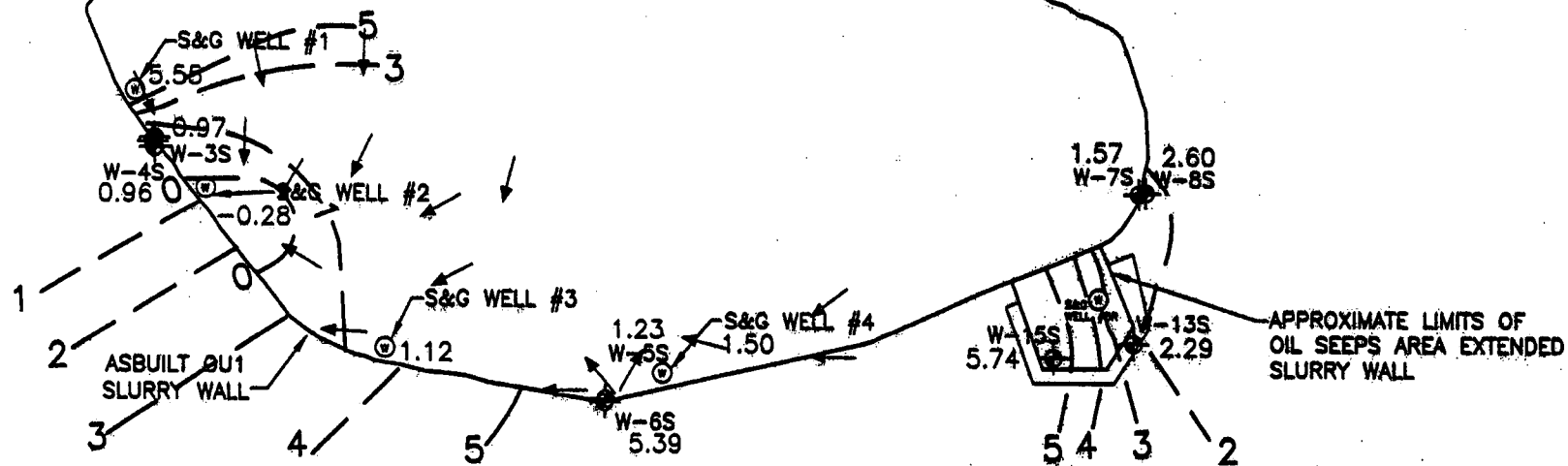
MONITORING WELL

1.53 WATER LEVEL ELEVATION IN SAND & GRAVEL MONITORING WELL IN FEET MSL, 12/23/02

2 SAND & GRAVEL POTENTIOMETRIC SURFACE IN FEET MSL, 12/23/02

BOUNDARY OF SAND & GRAVEL UNIT

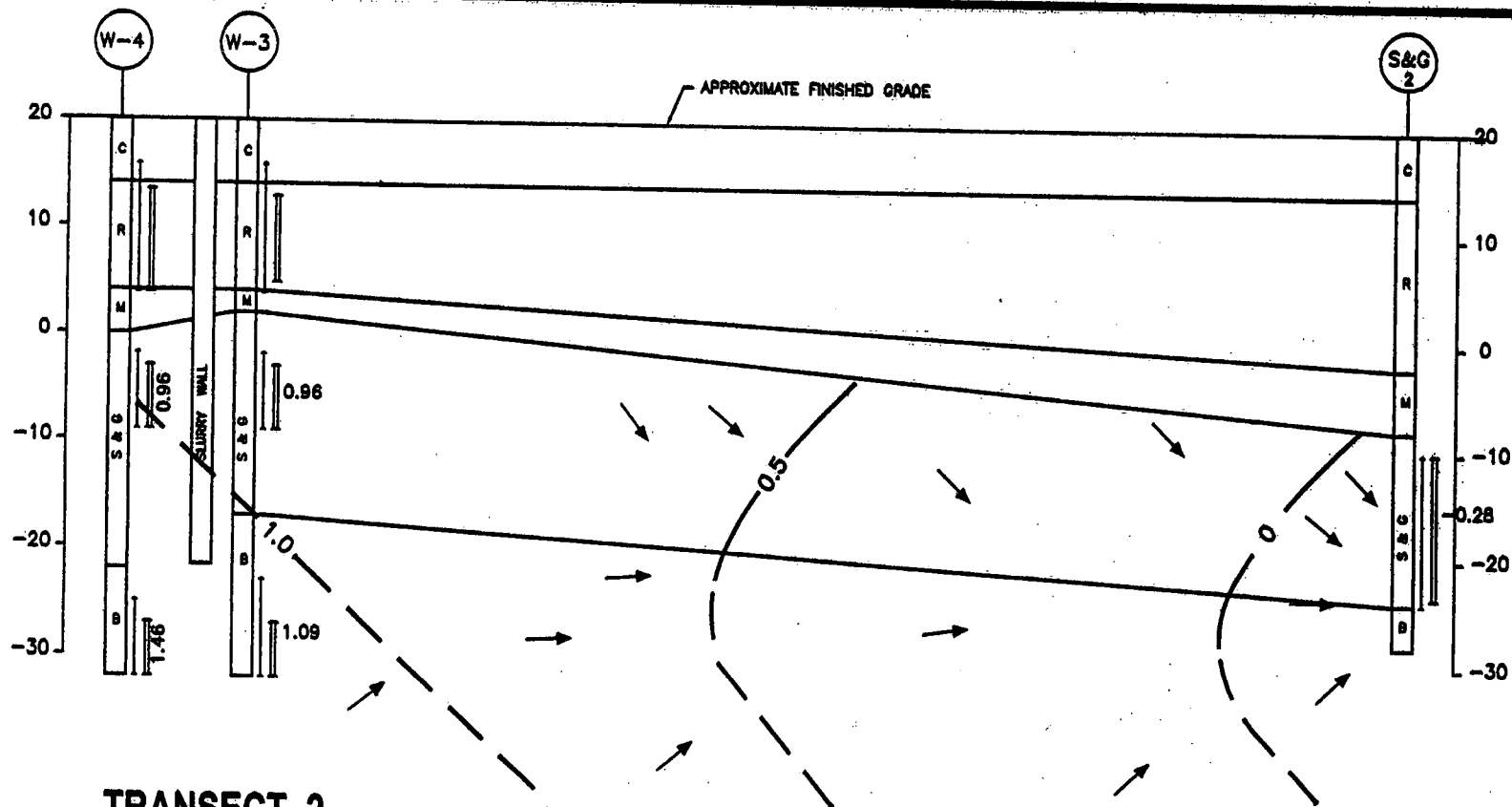
POTENTIAL LATERAL DIRECTION OF GROUNDWATER FLOW



0 400 800
 SCALE IN FEET

DATE _____
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 APP _____
 REV _____
 PROJECT NO.
 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
FIGURE 1
SAND & GRAVEL UNIT
POTENTIOMETRIC SURFACE



TRANSECT 2

SCALE: N.T.S.

LEGEND:

- B - BEDROCK
- C - CAP
- R - REFUSE
- M - MEADOW MAT
- S & G - SAND & GRAVEL

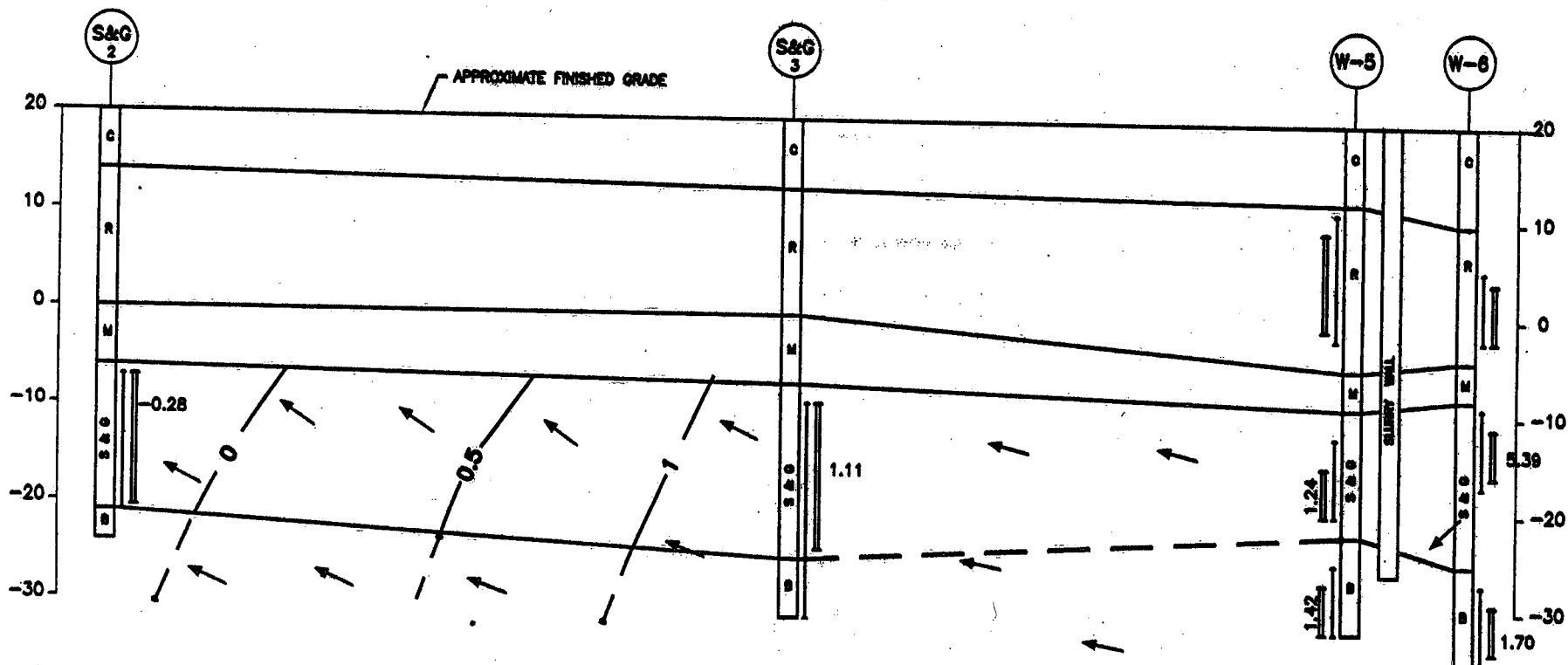
- 0.13 ELEVATION OF WATER LEVEL IN MONITORING WELL, IN FEET MSL., 12/23/02
- 0.5 GROUNDWATER EQUIPOTENTIAL IN BEDROCK AND SAND & GRAVEL, IN FEET MSL., 12/23/02
- POTENTIAL VERTICAL DIRECTION OF GROUNDWATER FLOW.

- SAND PACK INTERVAL
- SCREENED INTERVAL



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO.
 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
 FIGURE 2
 TRANSECT 2
 HYDROGEOLOGIC CROSS SECTION ANALYSIS



TRANSECT 3

SCALE: N.T.S.

LEGEND:

- B - BEDROCK
- C - CAP
- R - REFUSE
- M - MEADOW MAT
- S & G - SAND & GRAVEL

1.41

ELEVATION OF WATER LEVEL IN MONITORING WELL, IN FEET MSL, 12/23/02

-2

GROUNDWATER EQUIPOTENTIAL IN BEDROCK AND SAND & GRAVEL, IN FEET MSL, 12/23/02

→ POTENTIAL VERTICAL DIRECTION OF GROUNDWATER FLOW.

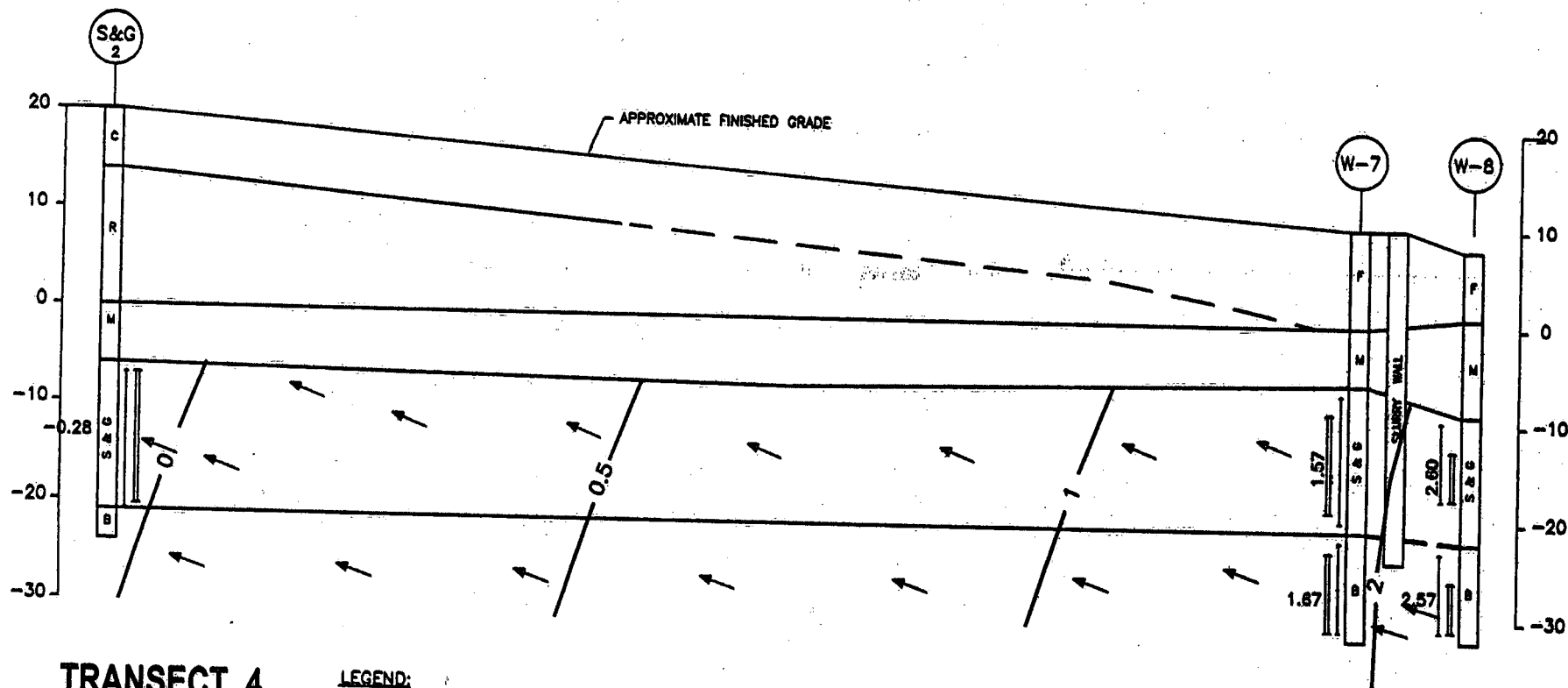
SAND PACK INTERVAL

SCREENED INTERVAL



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO. 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
 FIGURE 3
 TRANSECT 3
 HYDROGEOLOGIC CROSS SECTION ANALYSIS

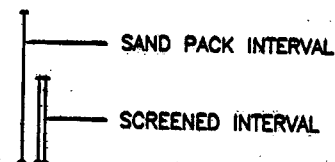


TRANSECT 4

SCALE: N.T.S.

LEGEND:

- B - BEDROCK 2.31 ELEVATION OF WATER LEVEL IN MONITORING WELL, IN FEET MSL, 12/23/02
- C - CAP -2- GROUNDWATER EQUIPOTENTIAL IN BEDROCK AND SAND & GRAVEL, IN FEET MSL, 12/23/02
- R - REFUSE
- M - MEADOW MAT
- S & G - SAND & GRAVEL → POTENTIAL VERTICAL DIRECTION OF GROUNDWATER FLOW.



DATE _____
 DWN _____
 APP _____
 REV _____
 PROJECT NO. 791186

KIN-BUC LANDFILL
 EDISON TOWNSHIP, EDISON, NEW JERSEY
 MIDDLESEX COUNTY, NEW JERSEY
 FIGURE 4
 TRANSECT 4
 HYDROGEOLOGIC CROSS SECTION ANALYSIS

Table 1
KinBuc Landfill Operable Units 1 and 2
Continuous Hydraulic Monitoring Results
2002 Minimum/Maximum/Average Water Elevations

Inside Slurry Wall					Outside Slurry Wall				
Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)	Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)
W-1G	October	11.86	12.18	12.00	W-2G	October	10.28	12.48	11.19
	November	11.57	11.86	11.73		November	10.29	13.36	12.10
	December	11.23	11.61	11.34		December	11.49	13.26	12.59
	4th Quarter	11.23	12.18	11.68		4th Quarter	10.28	13.36	12.00
W-3G	October	9.77	10.27	9.98	W-4G	October	10.78	11.57	11.11
	November	9.87	10.35	9.94		November	3.92	11.80	11.18
	December	9.57	10.31	9.89		December	10.84	11.69	11.23
	4th Quarter	9.57	10.35	9.93		4th Quarter	3.92	11.80	11.17
W-3S	October	0.14	1.83	0.79	W-4S	October	-0.37	2.84	1.09
	November	-9.45	1.91	0.88		November	-11.47	2.83	0.78
	December	-0.64	1.75	0.38		December	-1.15	3.17	0.62
	4th Quarter	-9.45	1.91	0.61		4th Quarter	-11.47	3.17	0.62
W-5G	October	4.18	4.93	4.38	W-6G	October	12.77	14.32	13.34
	November	4.07	4.80	4.39		November	2.84	13.86	13.33
	December	3.98	4.89	4.27		December	12.63	14.23	13.37
	4th Quarter	3.98	4.93	4.35		4th Quarter	2.84	14.32	13.35
W-5S	October	0.94	2.88	1.64	W-6S	October	5.16	6.95	5.78
	November	0.90	2.55	1.47		November	0.00	6.56	5.55
	December	0.28	2.72	1.27		December	4.33	6.86	5.45
	4th Quarter	0.28	2.72	1.42		4th Quarter	0.00	6.86	5.59
W-7S	October	1.35	2.85	1.82	W-8S	October	1.97	4.68	2.89
	November	1.34	2.54	1.78		November	-3.50	4.80	2.44
	December	0.78	2.80	1.53		December	1.88	5.50	2.41
	4th Quarter	0.78	2.85	1.70		4th Quarter	-3.50	5.50	2.51
W-15S	October	5.48	7.11	5.95	W-13S	October	1.88	3.78	2.41
	November	5.45	6.93	5.85		November	-4.33	3.73	2.22
	December	5.02	6.05	5.69		December	1.40	4.25	2.17
	4th Quarter	5.02	7.11	5.83		4th Quarter	-4.33	4.25	2.27
W-15G	October	1.02	1.24	1.11	W-13G	October	NA (1)	NA (1)	6.88 (2)
	November	-0.28	1.32	1.05		November	NA (1)	NA (1)	6.62 (2)
	December	0.88	1.35	1.09		December	2.68	3.51	2.97
	4th Quarter	-0.28	1.35	1.08		4th Quarter	2.68	3.51	2.97
W-9G	October	7.54	7.87	7.71	W-10G	October	8.57	8.72	8.65
	November	7.84	7.87	7.64		November	5.56	8.72	7.22
	December	7.18	7.70	7.34		December	5.73	8.11	7.63
	4th Quarter	7.18	7.87	7.58		4th Quarter	5.56	8.72	7.84
W-3RR	October	-0.37	1.97	0.70	W-4R	October	-0.08	3.37	1.58
	November	-14.21	2.38	0.66		November	-14.83	3.47	1.41
	December	-1.13	2.22	0.29		December	-0.62	3.77	1.17
	4th Quarter	-14.21	2.38	0.51		4th Quarter	-14.83	3.77	1.38

Table 1
KinBuc Landfill Operable Units 1 and 2
Continuous Hydraulic Monitoring Results
2002 Minimum/Maximum/Average Water Elevations

Inside Slurry Wall					Outside Slurry Wall				
Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)	Well ID	Monitoring Period	Minimum Recorded Water Elevation (ft)	Maximum Recorded Water Elevation (ft)	Average Water Elevation (ft)
W-5R	October	1.08	2.78	1.67	W-6R	October	1.27	2.93	1.83
	November	-10.59	2.58	1.49		November	-4.23	2.86	1.72
	December	0.28	2.89	1.34		December	0.67	3.21	1.71
	4th Quarter	-10.59	2.89	1.51		4th Quarter	-4.23	3.21	1.75
W-7R	October	1.43	2.92	1.89	W-8RR	October	1.99	4.72	2.72
	November	-7.21	2.52	1.75		November	-16.12	4.82	2.45
	December	0.81	2.88	1.60		December	1.67	5.60	2.48
	4th Quarter	-7.21	2.92	1.74		4th Quarter	1.67	5.60	2.48

Notes:

1. Troll malfunctioned, data was not collected.
2. Water elevation calculated from manual water levels.

Table 2-5
KinBuc Landfill Operable Unit 1
Fourth Quarter 2002
Troll Water Elevations vs. Manual Water Elevations

OU 1 Well ID	November 6, 2002			December 6, 2002			January 2, 2003			Average
	Troll	Manual	Difference	Troll	Manual	Difference	Troll	Manual	Difference	Difference
W-1G	11.87	11.81	0.06	11.50	11.49	0.01	11.28	11.27	0.01	0.03
W-2G	11.27	11.27	0.00	12.37	12.38	0.01	13.30	13.31	0.01	0.01
W-3G	9.34	9.29	0.05	8.01	7.96	0.05	7.97	7.92	0.05	0.05
W-3S	1.28	1.31	0.03	0.32	0.33	0.01	1.20	1.20	0.00	0.01
W-3RR	1.50	1.50	0.00	0.54	0.54	0.00	1.45	1.45	0.00	0.00
W-4G	11.43	11.42	0.01	11.10	11.11	0.01	11.19	11.18	0.01	0.01
W-4S	2.56	2.05	0.51	1.67	1.67	0.00	2.54	2.54	0.00	0.17
W-4R	2.62	2.56	0.06	1.63	1.57	0.06	2.44	2.37	0.07	0.06
W-5G	10.38	10.40	0.02	9.92	9.92	0.00	9.77	9.82	0.05	0.02
W-5S	2.12	2.16	0.04	1.24	1.17	0.07	2.19	2.21	0.02	0.04
W-5R	1.92	1.96	0.04	1.09	1.10	0.01	2.05	2.08	0.03	0.03
W-6G	13.84	13.82	0.02	13.36	13.32	0.04	13.61	13.63	0.02	0.03
W-6S	2.38	2.43	0.05	1.43	1.45	0.02	2.36	2.40	0.04	0.04
W-6R	2.44	2.44	0.00	1.45	1.35	0.10	2.46	2.48	0.02	0.04
W-7S	2.29	2.31	0.02	1.43	1.43	0.00	2.15	2.18	0.03	0.02
W-7R	2.43	2.45	0.02	1.50	1.49	0.01	2.21	2.21	0.00	0.01
W-8S	2.61	2.63	0.02	2.38	2.38	0.00	2.59	2.60	0.01	0.01
W-8RR	2.62	2.60	0.02	2.37	2.35	0.02	2.60	2.54	0.06	0.03
W-9G	7.87	7.89	0.02	7.33	7.35	0.02	7.34	7.36	0.02	0.02
W-10G	8.70	8.70	0.00	6.84	6.85	0.01	8.12	8.04	0.08	0.03
W-13G	NA (1)	6.88	NA (1)	NA (1)	6.62	NA (1)	6.85	6.82	0.03	0.03
W-13S	2.55	2.54	0.01	1.99	1.99	0.00	2.45	2.44	0.01	0.01
W-15G	1.61	1.61	0.00	1.52	1.52	0.00	1.51	1.51	0.00	0.00
W-15S	2.72	2.69	0.03	2.13	2.15	0.02	2.65	2.69	0.04	0.03

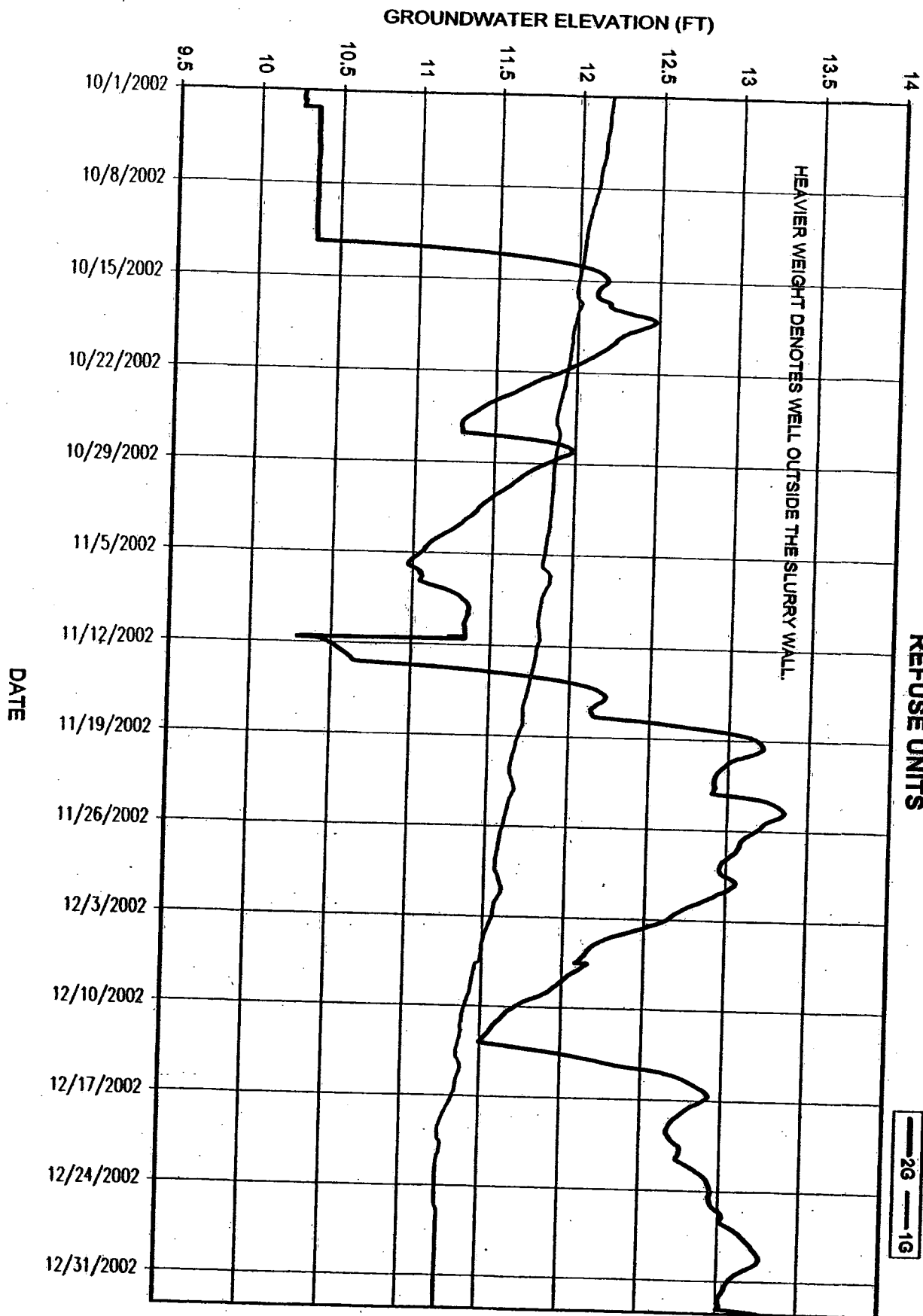
Notes : (1) Troll data was not collected due to device malfunction. Water levels taken manually.

Table 2-6
Kin-Buc Landfill
Leachate Cleanout Monitoring
2002

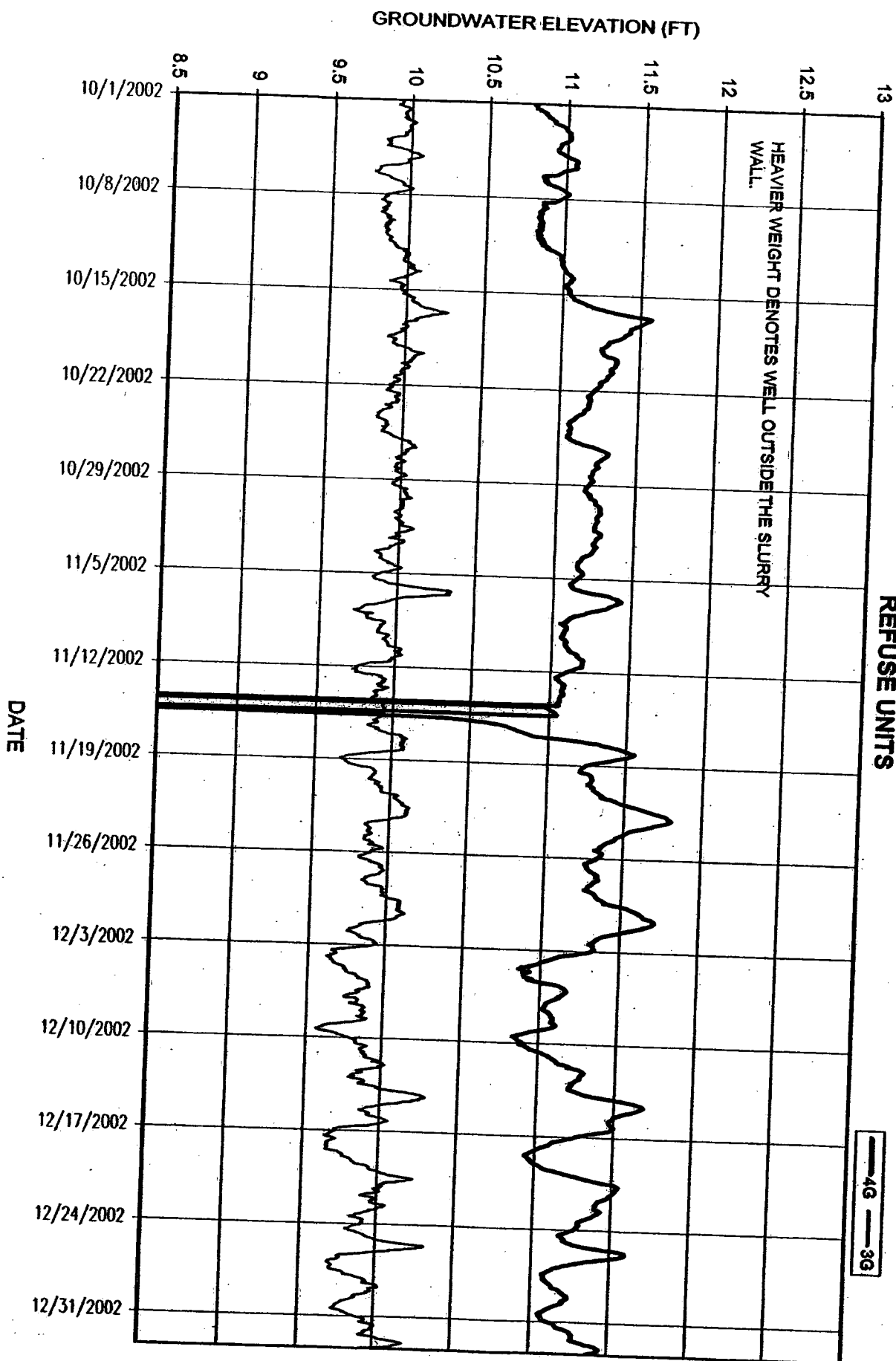
Cleanout location	14N		14E		15N		15E		16N		16E	
Elevation @ Sea Level	22.87		22.77		26.51		26.51		31.36		31.32	
	depth to water	elevation	depth to water	elevation	depth to water	elevation	depth to water	elevation	depth to water	elevation	depth to water	elevation
Elevation Average		10.09		10.06		9.85		9.93		na		na
DATE												
12/10/2001	12.5	10.37	12.42	10.35	16.31	10.20	16.33	10.18	dry	na	dry	na
1/3/2002	12.37	10.50	12.31	10.46	16.21	10.30	16.22	10.29	dry	na	dry	na
2/13/2002	12.70	10.17	12.63	10.14	16.57	9.94	16.62	9.89	dry	na	dry	na
3/27/2002	12.61	10.26	12.55	10.22	16.52	9.99	16.47	10.04	dry	na	dry	na
4/19/2002	12.75	10.12	12.68	10.09	16.64	9.87	16.61	9.90	dry	na	dry	na
5/3/2002	13.03	9.84	12.96	9.81	16.97	9.54	16.94	9.57	dry	na	dry	na
6/5/2002	13.04	9.83	12.97	9.80	16.63	9.88	16.95	9.56	dry	na	dry	na
7/8/2002	12.86	10.01	12.79	9.98	16.77	9.74	16.72	9.79	dry	na	dry	na
8/2/2002	12.86	10.01	12.79	9.98	16.8	9.71	15.73	10.78	dry	na	dry	na
9/5/2002	12.86	10.01	12.78	9.99	16.77	9.74	16.75	9.76	dry	na	dry	na
9/26/2002	12.94	9.93	12.85	9.92	16.85	9.66	16.83	9.68	dry	na	dry	na
11/6/2002	12.64	10.23	12.58	10.19	16.59	9.92	16.48	10.03	dry	na	dry	na
12/6/2002	13.02	9.85	12.94	9.83	16.97	9.54	16.95	9.56	dry	na	dry	na
1/2/2003	13.07	9.80	13.00	9.77	17.03	9.48	17.01	9.50	dry	na	dry	na

ATTACHMENT 1

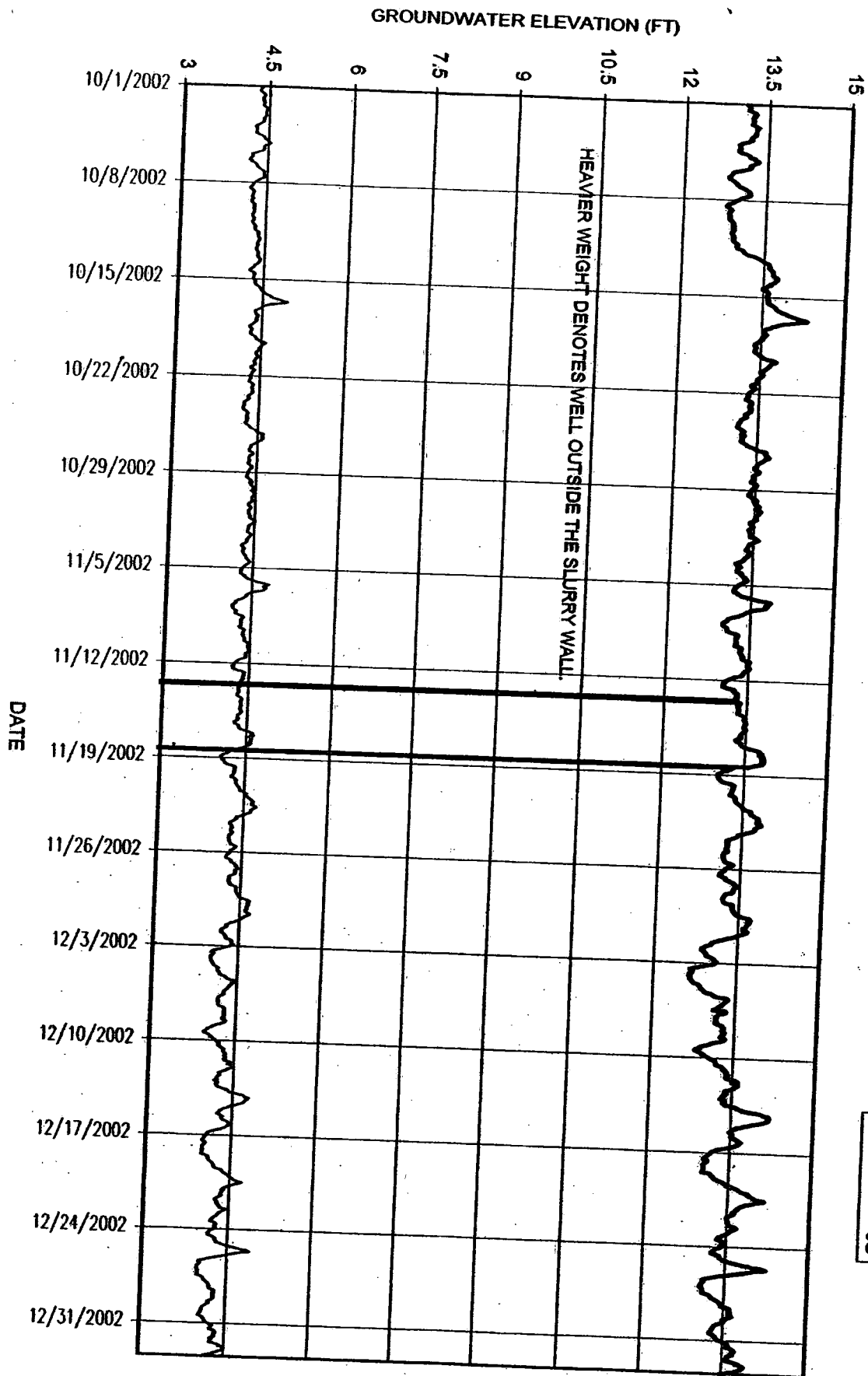
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #1 TRANSECT No. 1 REFUSE UNITS



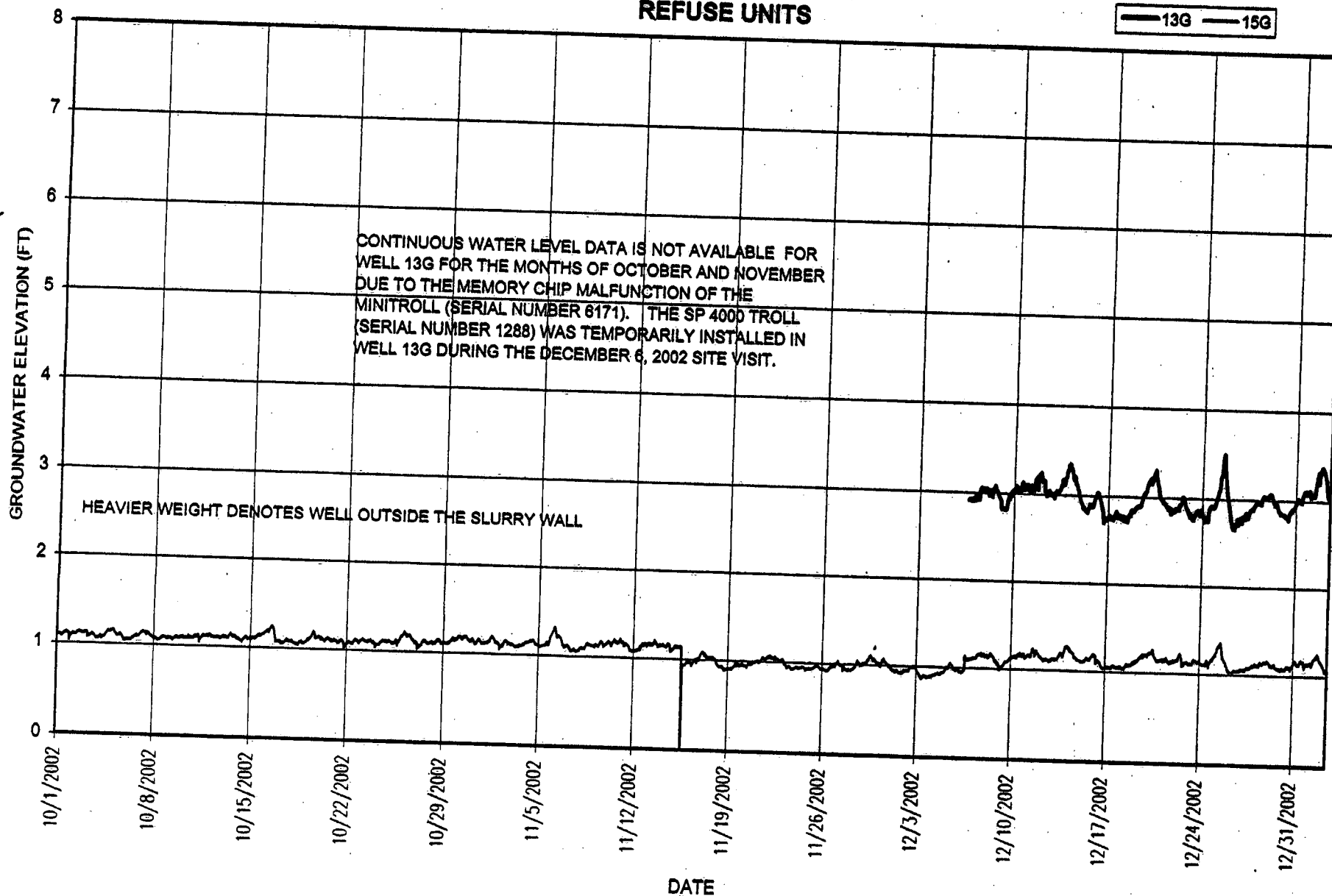
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #2 TRANSECT No.2 REFUSE UNITS



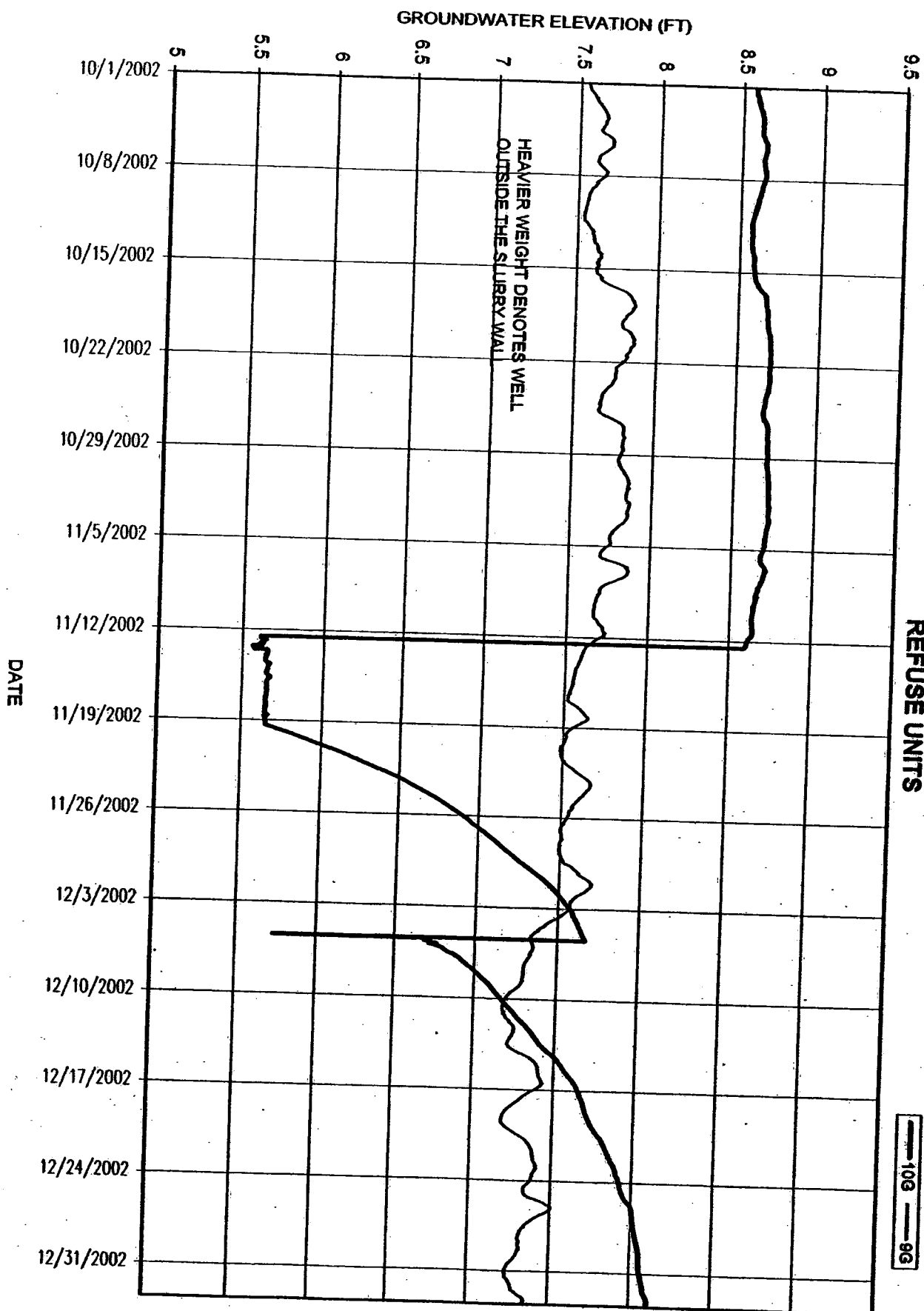
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH # 3
 TRANSECT No.3
 REFUSE UNITS



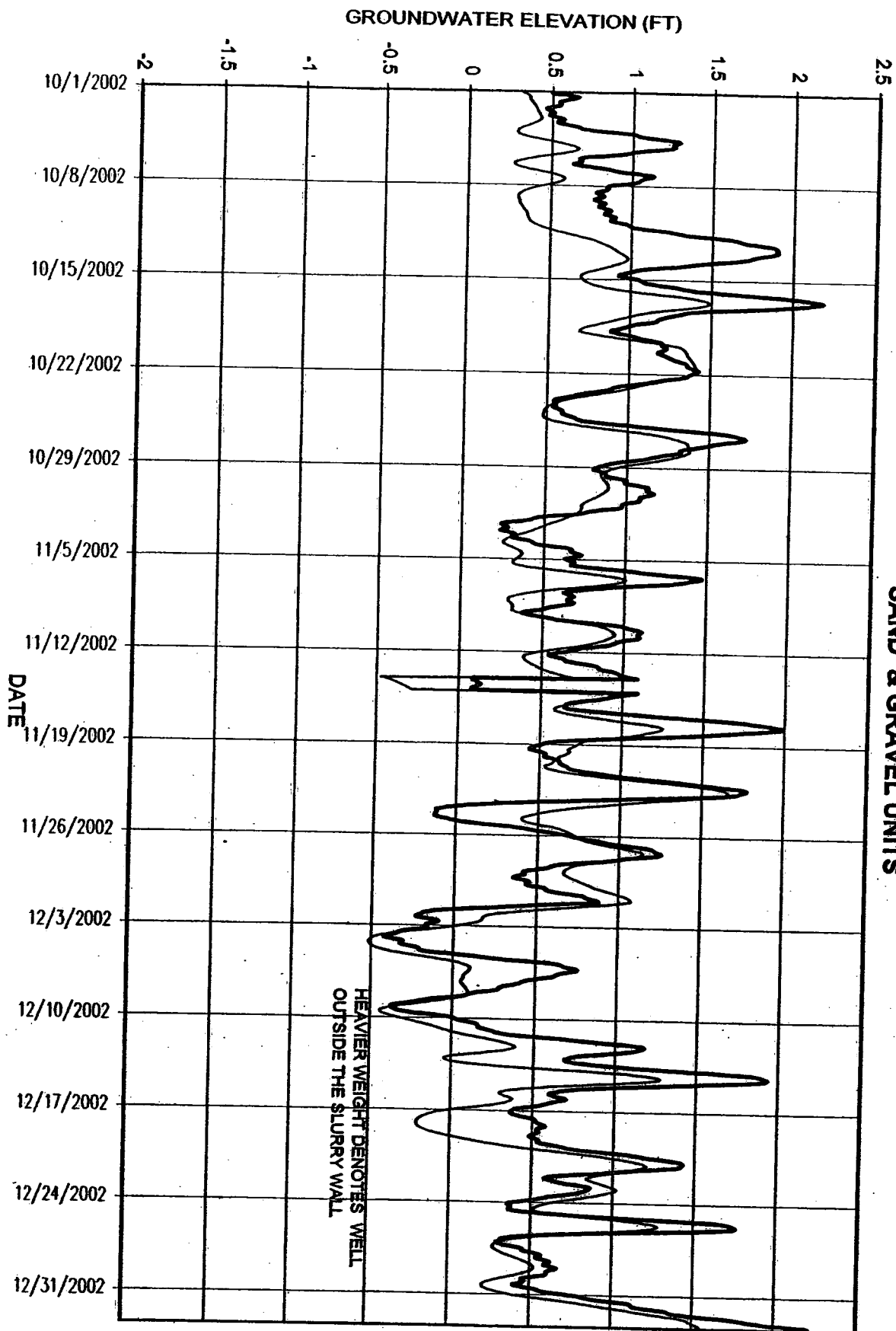
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #4 TRANSECT No.4 (OSA) REFUSE UNITS



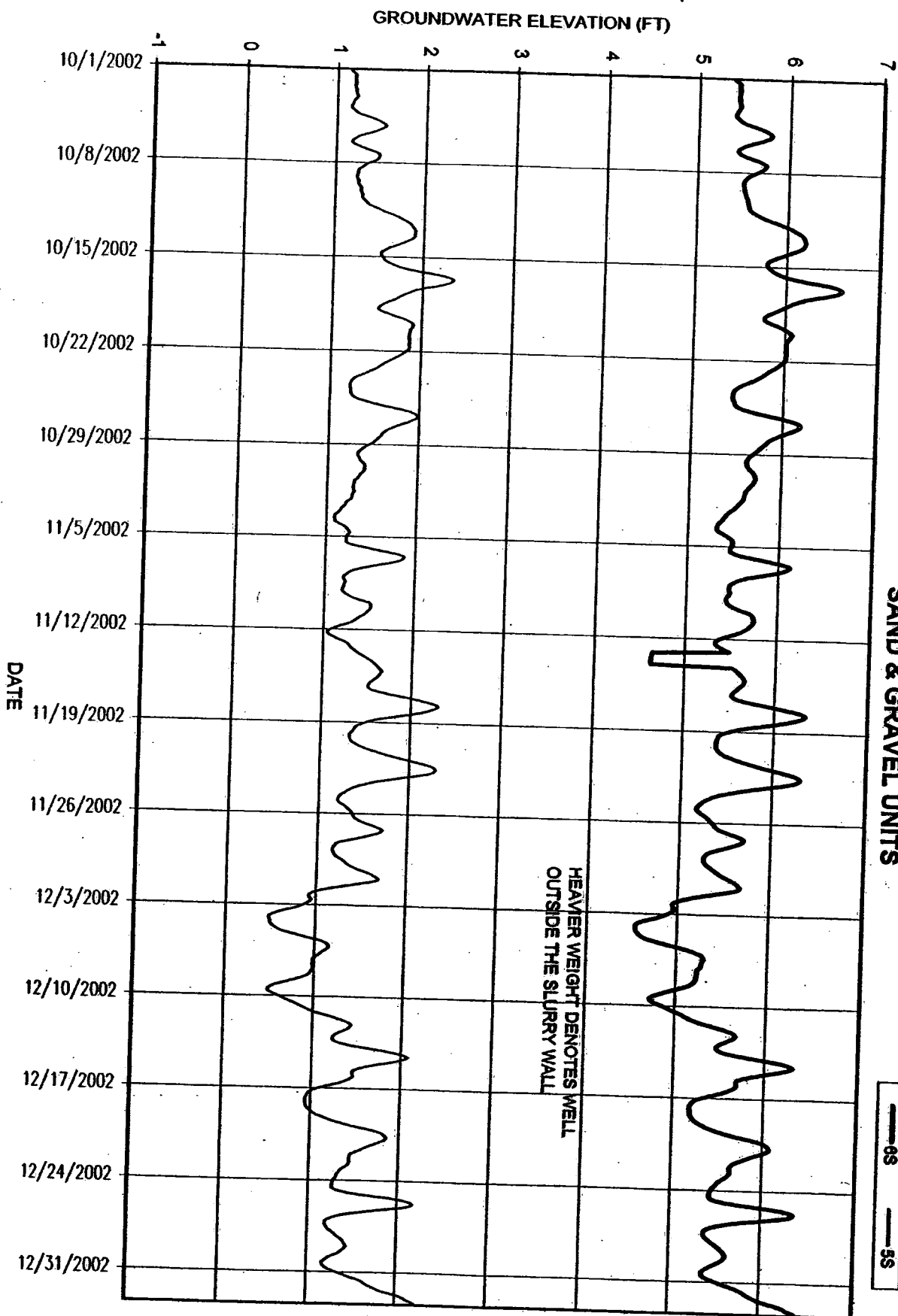
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #5 TRANSECT No.5 REFUSE UNITS



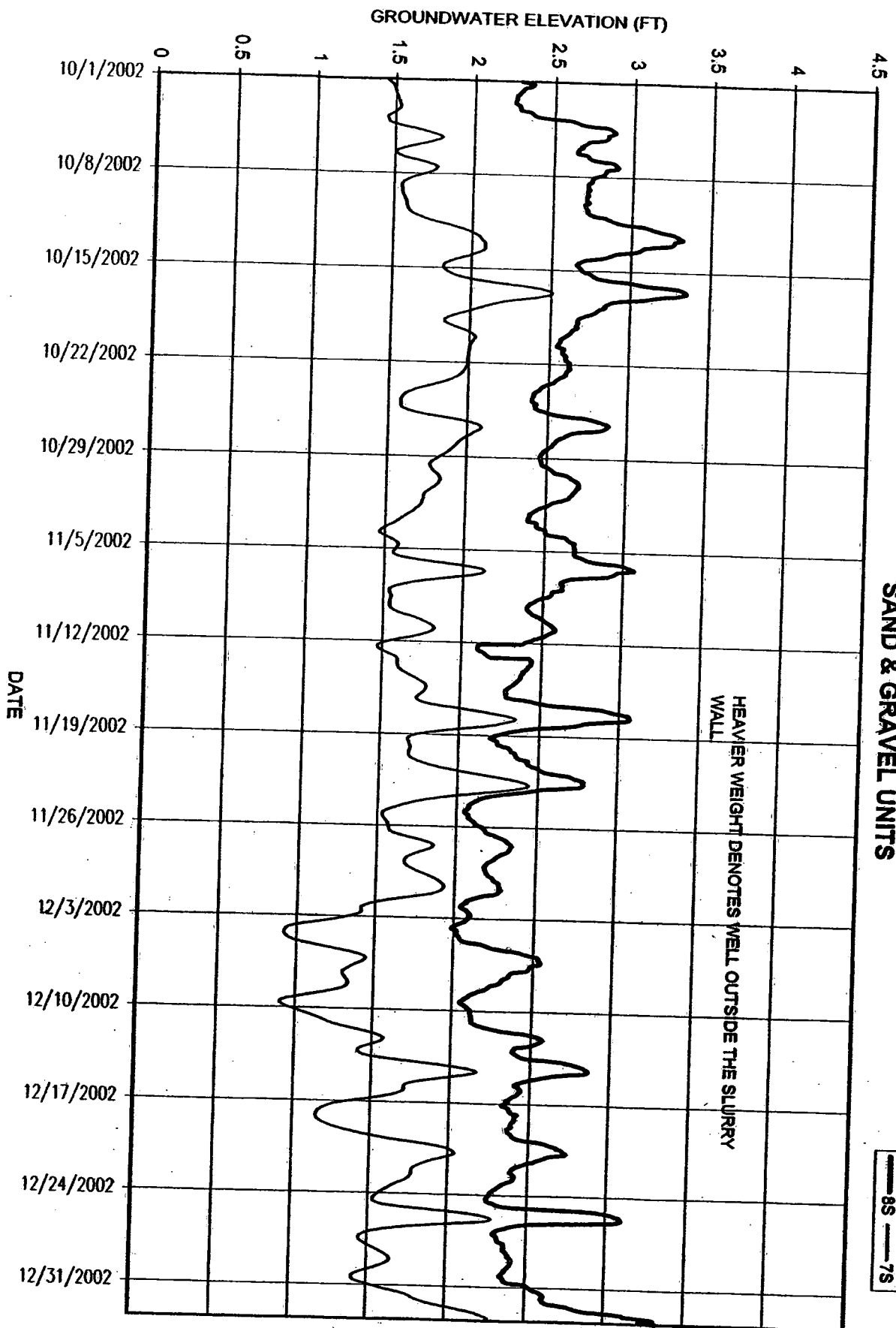
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #6
 TRANSECT No.2
 SAND & GRAVEL UNITS



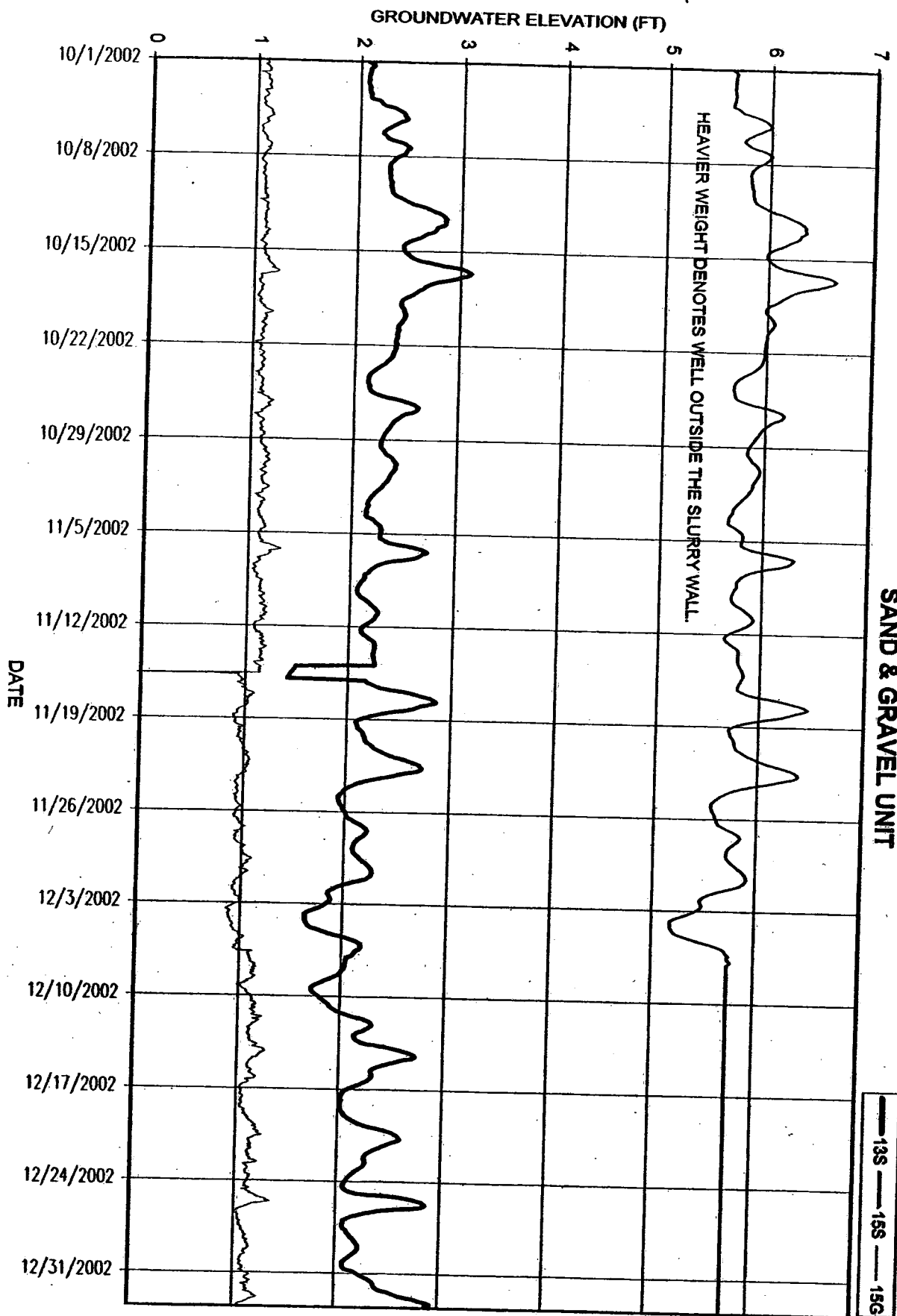
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #7 TRANSECT No.3 SAND & GRAVEL UNITS



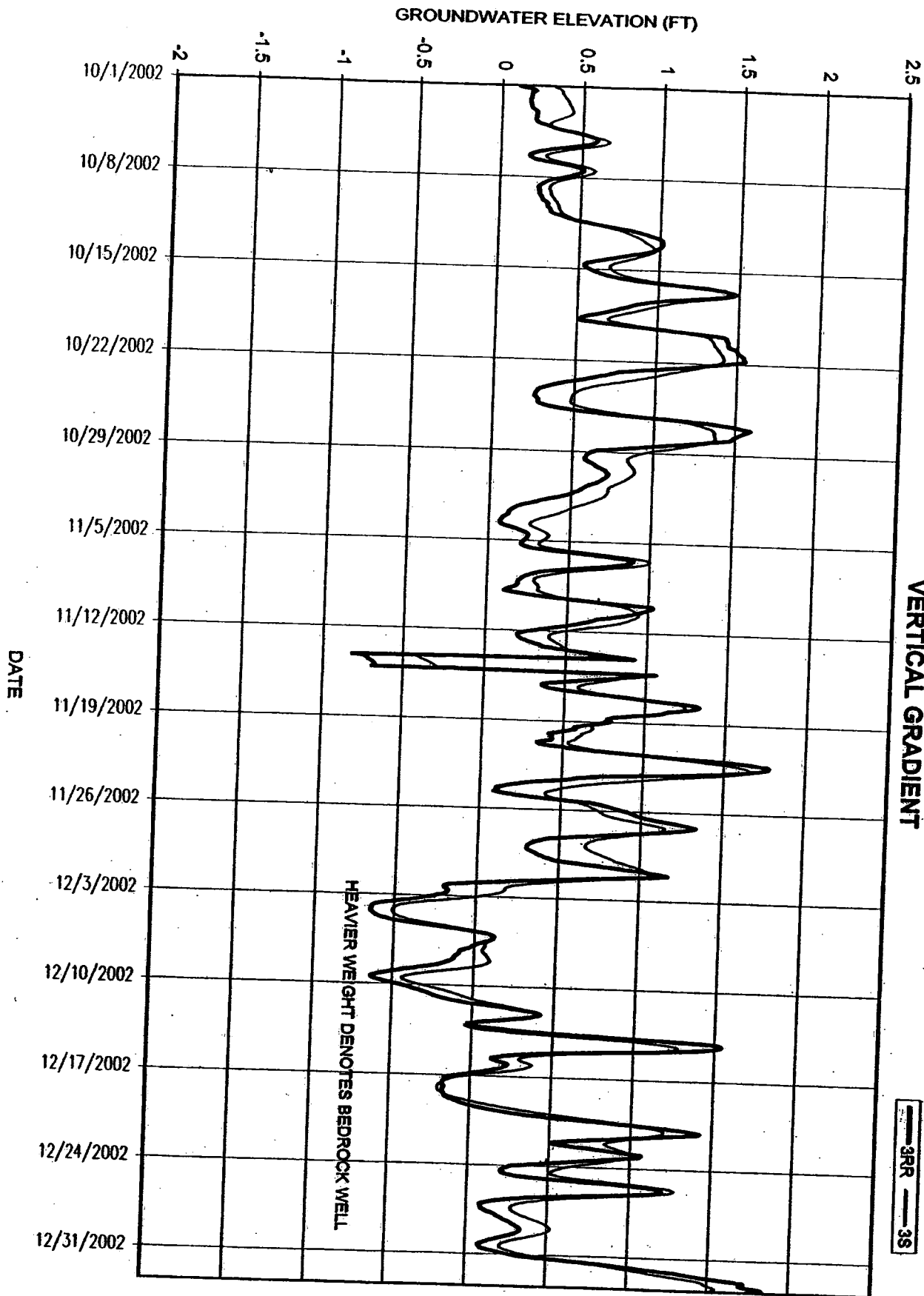
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #8
 TRANSECT NO.4
 SAND & GRAVEL UNITS



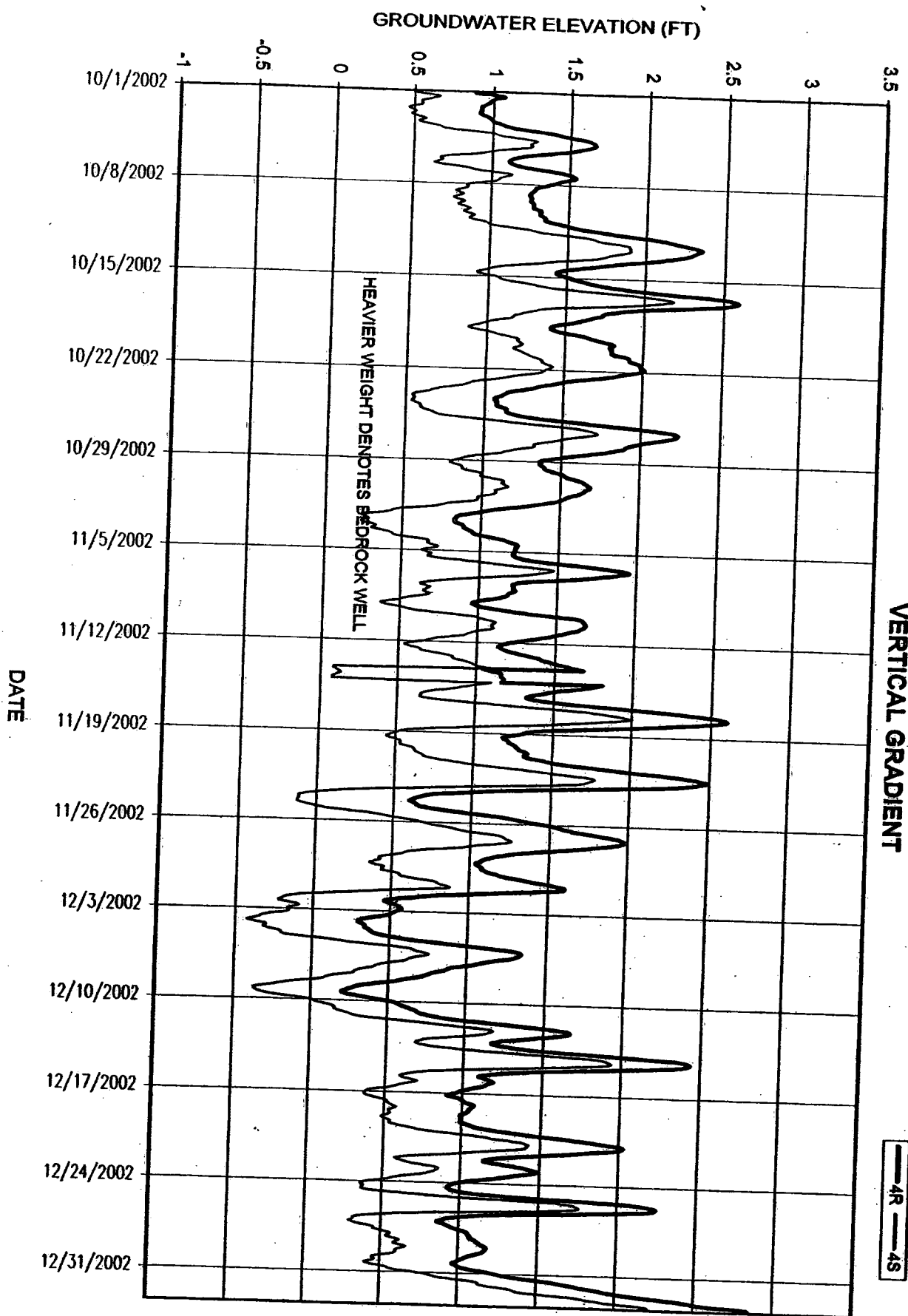
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #9 TRANSECT No.4 (OSA) SAND & GRAVEL UNIT



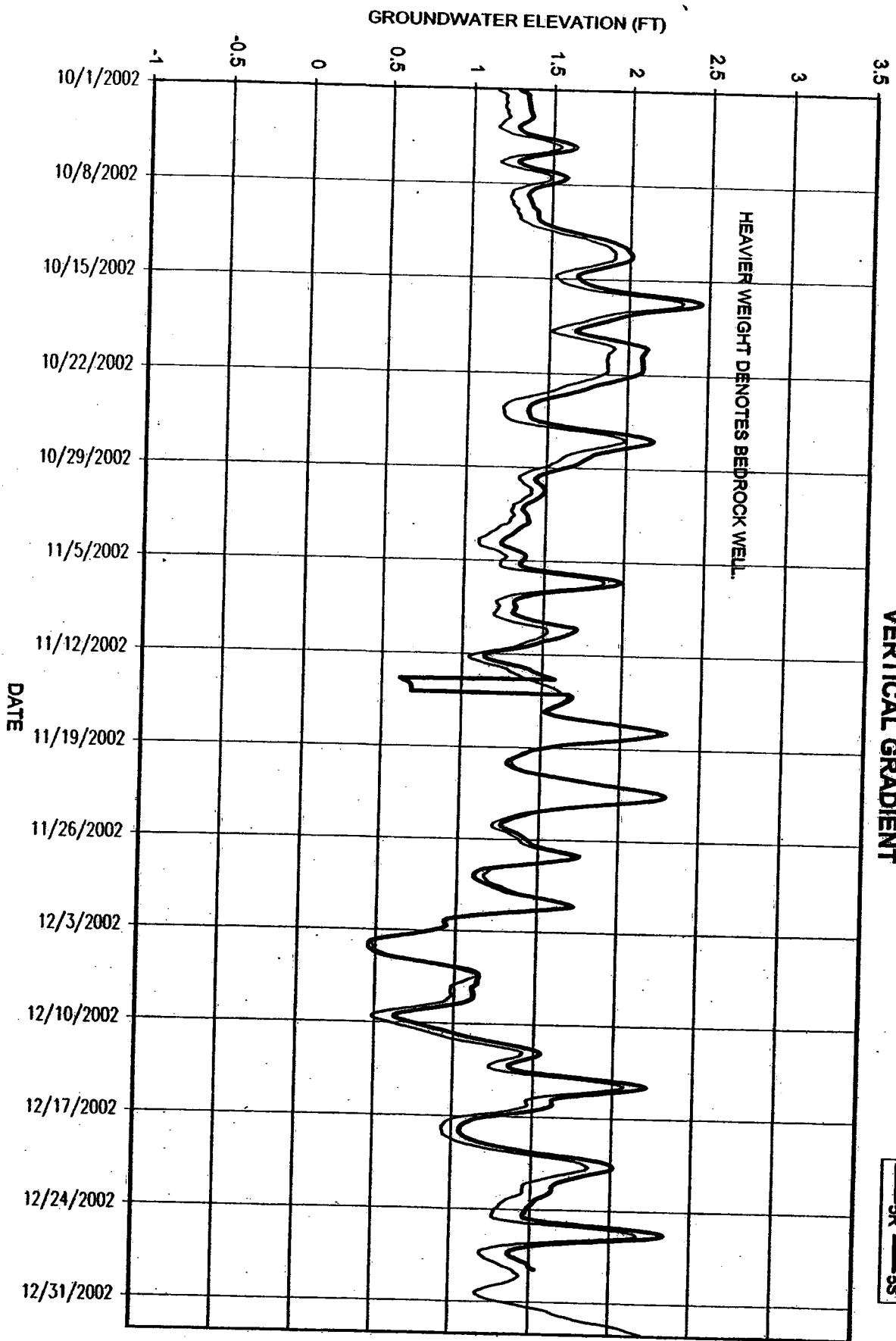
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #10 TRANSECT No.2 - INSIDE VERTICAL GRADIENT



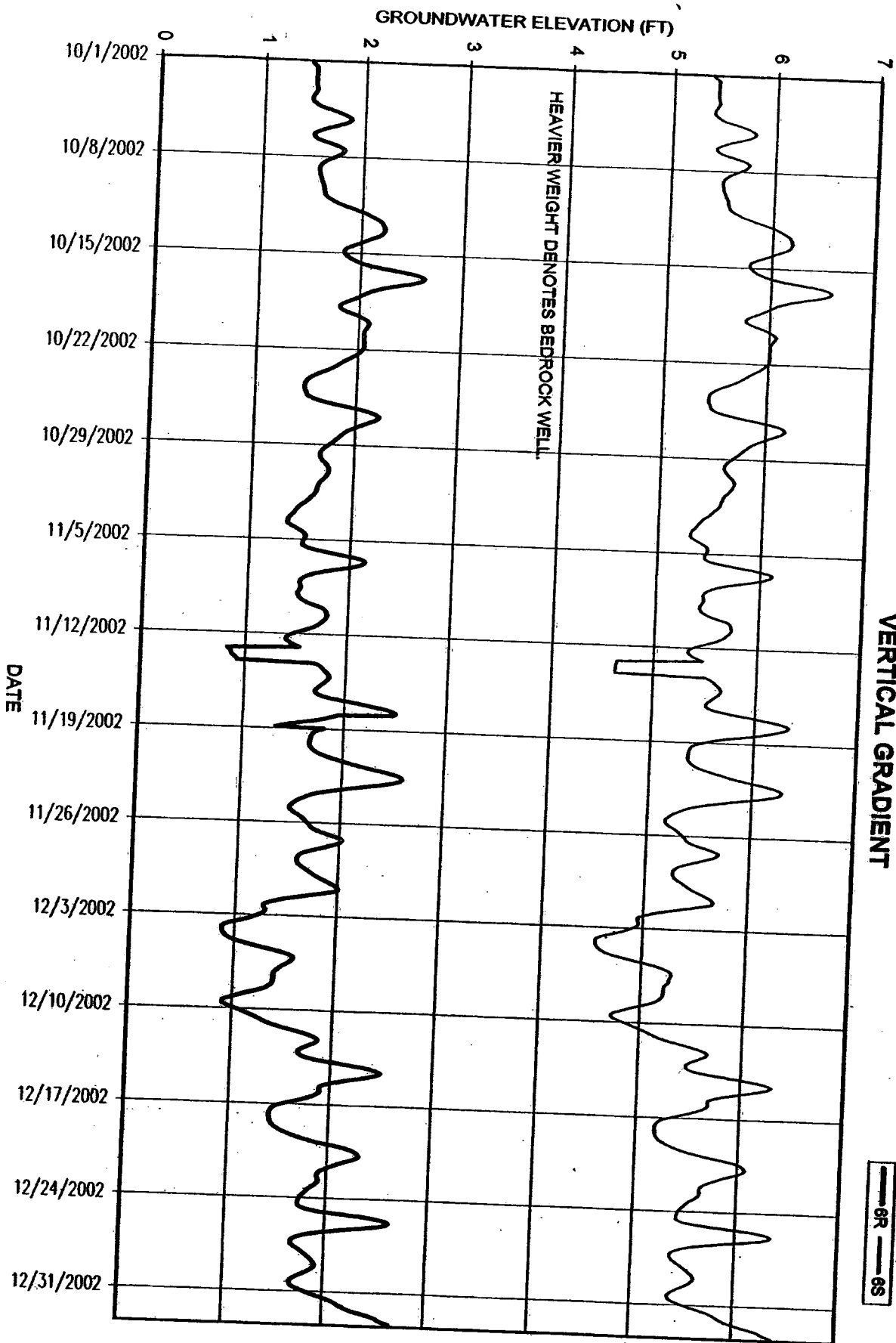
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #11 TRANSECT NO.2 - OUTSIDE VERTICAL GRADIENT



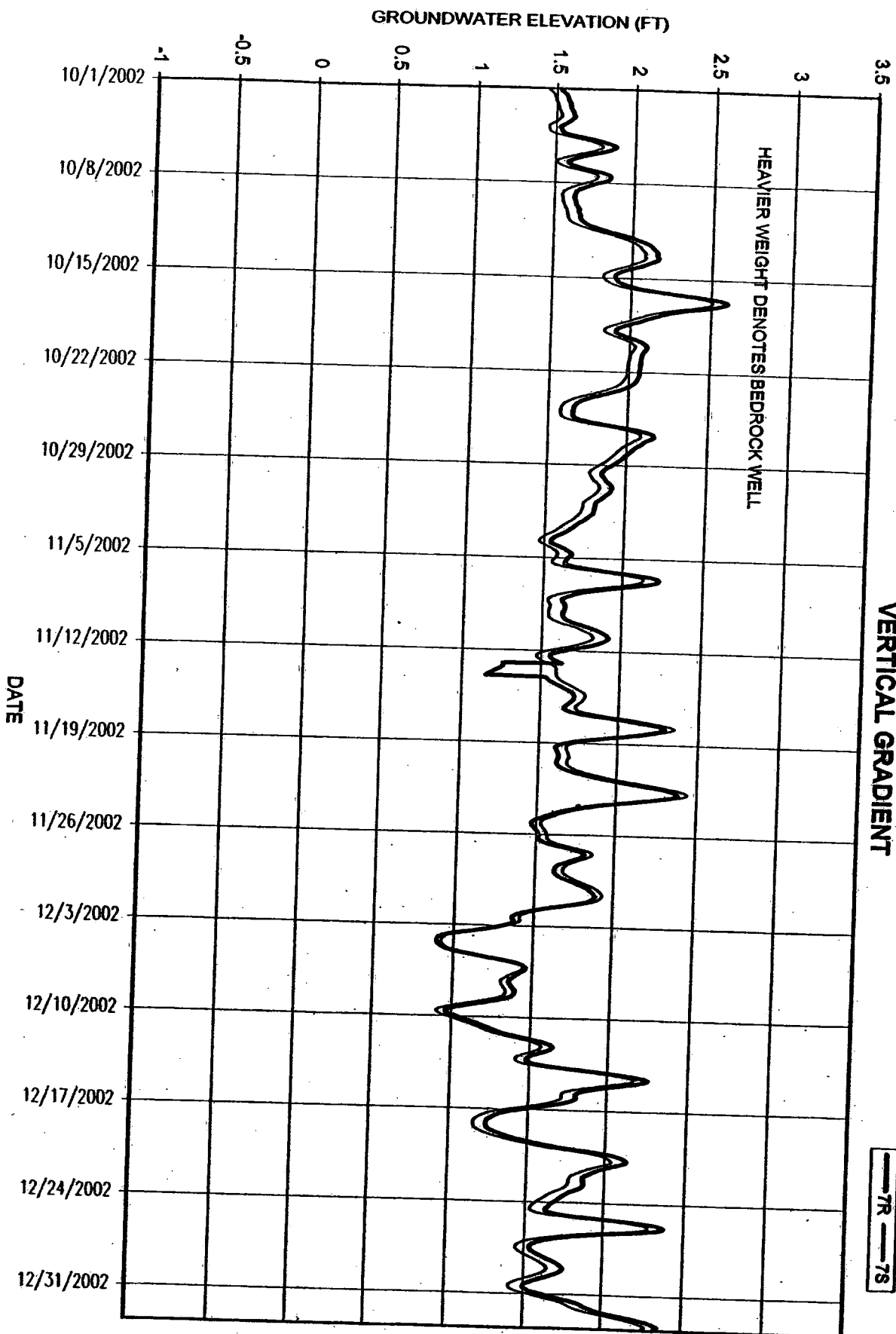
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #12 TRANSECT No.3 - INSIDE VERTICAL GRADIENT



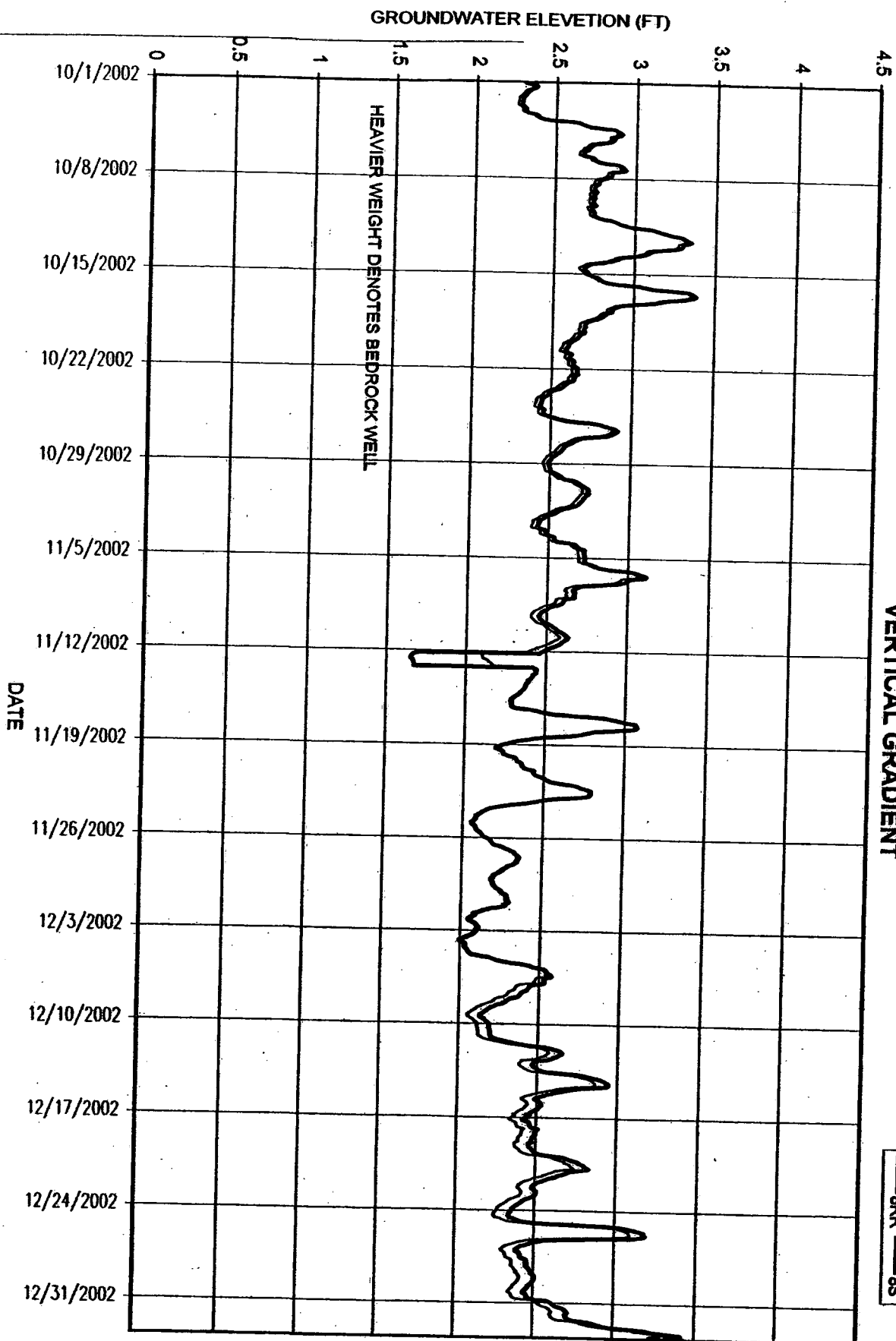
KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #13 TRANSECT No.3 - OUTSIDE VERTICAL GRADIENT



KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #14 TRANSECT No.4- INSIDE VERTICAL GRADIENT



KIN-BUC LANDFILL GROUNDWATER HYDROGRAPH #15 TRANSECT No.4- OUTSIDE VERTICAL GRADIENT



ATTACHMENT 2



IT Corporation

**Crossroads Corporate Center
One International Boulevard, Suite 700
Mahwah, NJ 07495-0086
Tel. 201.512.5700
Fax. 201.512.5786**

A Member of The IT Group

**June 27, 2001
Project 796201**

**Carl Januszkiewicz
Waste Management, Inc
Kin-Buc Landfill Treatment Plant
383 Meadow Road
Edison, NJ 08817**

Re: Eyaluation of Head Levels at Transect 1

Dear Mr. Januszkiewicz:

We have completed an evaluation of the hydraulic characteristics at Transect 1 with specific focus on the lack of intragradient conditions associated with the high water levels in W-1G (inside of the slurry wall) relative to those levels in W-2G (outside of the wall).

While intragradient conditions were evident at the outset of the hydraulic monitoring program in April 1996, these conditions have generally not been maintained. Specifically, based on a review of historical hydrographs, intragradient conditions were evident initially from approximately April to July 1996, and April to June 1997. Thereafter, to more recent events, intragradient conditions have been observed intermittently and for shorter periods of time.

Attachment 1 presents a hydrograph at Transect 1 encompassing the period from September 1998 to December 2000. As seen on the hydrograph, there were periods of time when intragradient conditions were not being maintained.

As opposed to the other "G" series monitoring wells that are located in refuse, wells 1G and 2G at Transect 1 are actually located in a silt and clay deposit. Attachment 2 contains the boring logs for these 2 installations. In-situ hydraulic conductivity testing performed at Transect 1 indicated permeabilities of 10^{-7} cm/sec and 10^{-5} cm/sec in W-1G and W-2G, respectively. Accordingly, a source of recharge to the overburden soils in the area of W-1G would not readily drain away, and therefore, higher heads could result.

Well 1G sampling events (November 1998, October 1999, October 2000) can be seen on the hydrograph as sharp vertical drops in groundwater levels. Due to the low permeability of the surrounding materials, the groundwater levels required several months to recover. Since the final cover extends 10 feet past the slurry wall, the source of the groundwater that is recharging W-1G is unknown at present.

The hydraulic gradient between W-1G and W-1R is vertically downward which rules out the bedrock as being a source of groundwater recharge. Based on a recent visual inspection of the area around Transect 1, the cap appears to be good condition and there were no signs that the cap integrity has been compromised.

Figure 1 depicts the conceptual model of the hydraulic interrelationship across Transect 1 showing water level measurements that depict the lack of intragradient conditions across the

Carl Januszkiewicz
June 27, 2001
Page 2

Project 796201

slurry wall. The head levels in W-2G (outside the slurry wall) are generally at elevation 12 to 13 feet msl with periodic and short term increases to about 15 feet msl. The water level in the well sometimes falls below the level of the transducer. This is characterized by a flat straight line on the hydrographs as shown on Attachment 1. Head levels in W-1G (inside the slurry wall), on the other hand, are often greater with elevations as high as 15 to 16 feet msl being recorded.

It is evident from a review of Figure 1 that the drop in topography outside of the slurry wall toward Mill Brook, coupled with the higher permeability of W-2G relative to W-1G, would promote a more rapid decrease of head levels in the latter. This suggests that intragradient conditions may not be consistently attainable at this transect in any event. This notwithstanding however, and as depicted on Figure 1, it is important to note that the leachate collection system represents a hydraulic sink within the containment system. As such, groundwater in the vicinity of W-1G would drain toward the sink mitigating concerns of outward flow.

The leachate collection line runs parallel to the slurry wall and at its closest point is only about 20 feet away from Transect 1. Several cleanouts are located along the collection line with the closest, Cleanout 16, only about 65 feet from Transect 1. Leachate level measurements obtained from the cleanouts during December 2000 and June 2001 indicate a leachate level of 10 to 11 feet msl along the collection line as shown in Table 1. The leachate levels observed suggest that the leachate collection system is presently operating effectively.

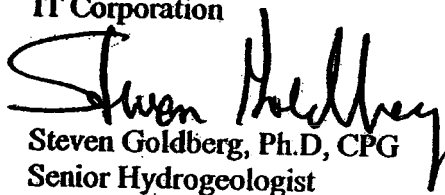
Recommendations

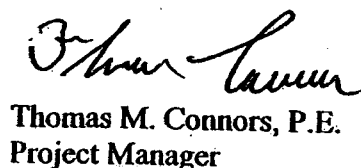
Based on the above, it is recommended that during subsequent monitoring events at the site, measurements of leachate levels in Cleanouts 14 through 16 be recorded to verify that the leachate collection system is operating effectively. If liquid levels in the cleanouts increase above 12 to 13 feet msl, then maintenance of the collection line is recommended. Subsequent reports to EPA should include a discussion of the leachate collection system and its role as serving as a hydraulic sink within the containment system.

We trust you find this information useful. If you have any questions, please do not hesitate to contact us.

Sincerely,

IT Corporation

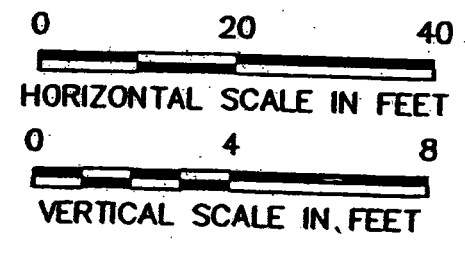
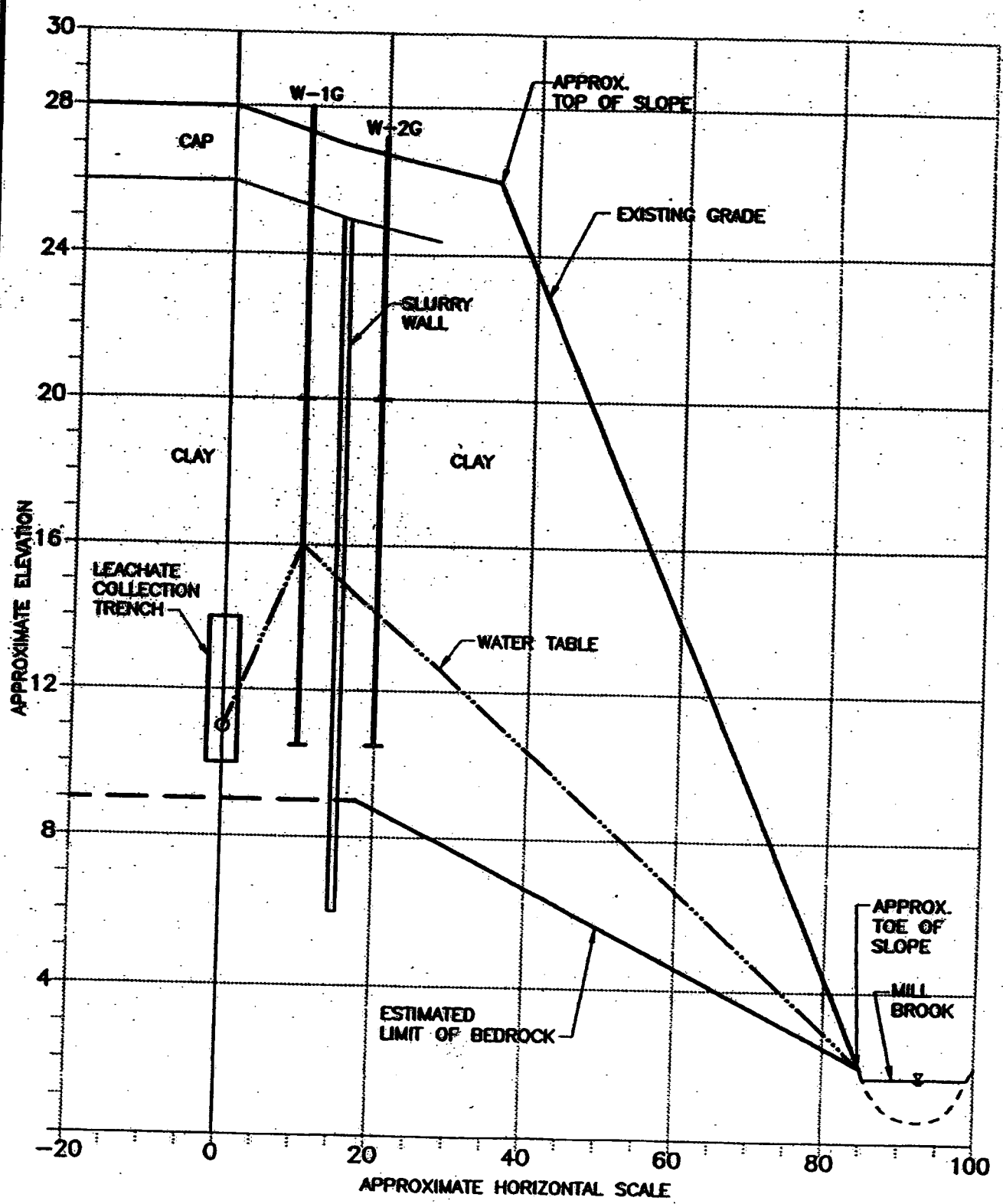

Steven Goldberg, Ph.D, CPG
Senior Hydrogeologist



Thomas M. Connors, P.E.
Project Manager

Attachments

Project: Kin-Buc Landfill
Drawing Title: Cross Section Transect 1
Drawing Number: 796201-A1
Date: 2/28/01
Scale: 1" = 20' Horizontal, 1" = 4' Vertical

IMAGE	X-REF	OFFICE	DRAWN BY	CHECKED BY	APPROVED BY	DRAWING NUMBER
		Metuchen, NJ	A. Krasinski	2/28/01		796201-A1





WASTE MANAGEMENT INC.

FIGURE 1
CROSS SECTION
TRANSECT 1
KIN-BUC LANDFILL
EDISON, NEW JERSEY

Table 1

Kin-Buc Landfill

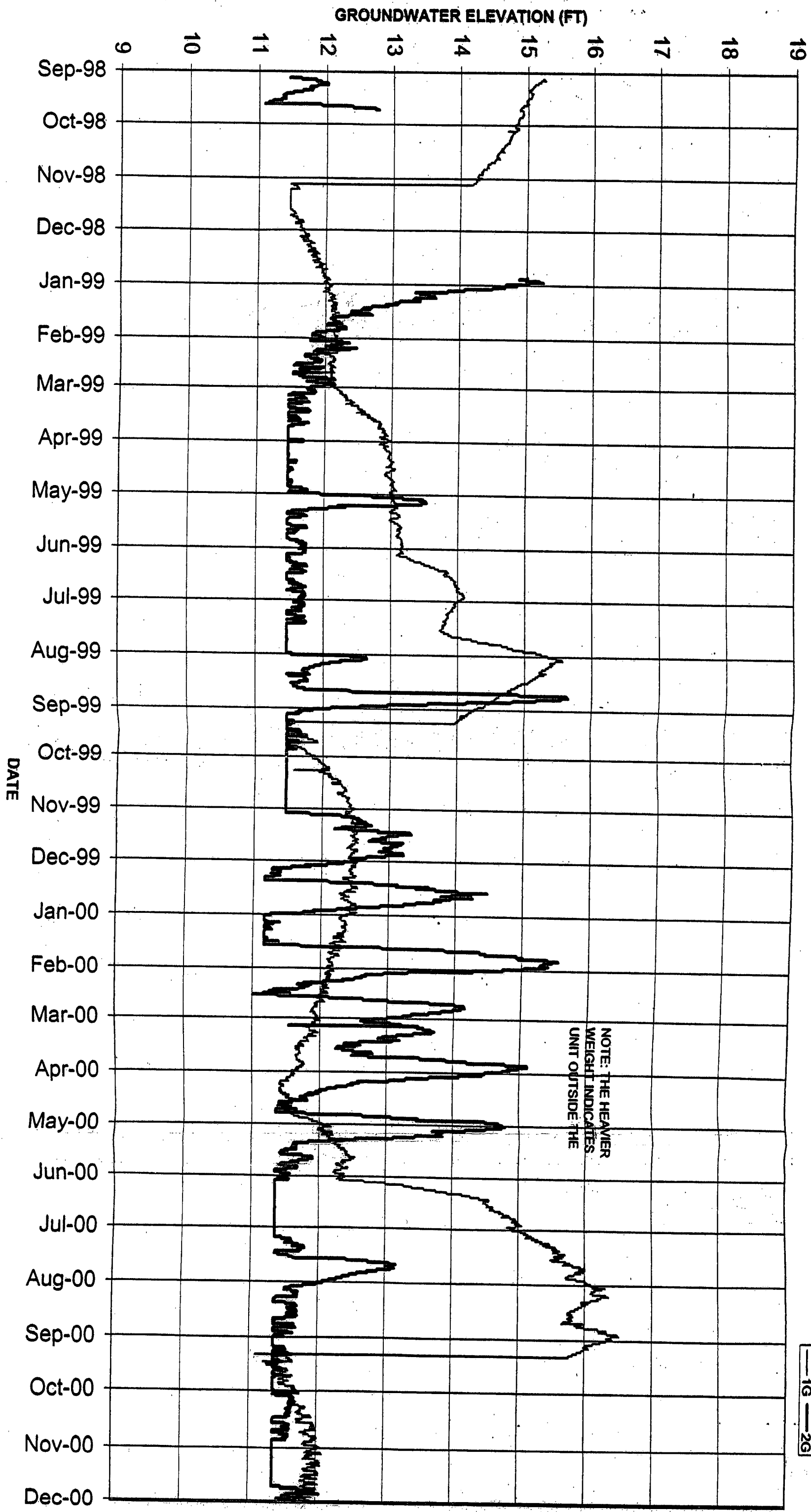
Leachate Cleanout Monitoring

2001

[illegible]

ATTACHMENT 1

KIN-BUC LF GROUNDWATER ELEVATION HYDROGRAPH AT TRANSECT NO.1 REFUSE UNIT



ATTACHMENT 2

MONITORING WELL RECORD

Well Permit No. 25 - 46506
 Atlas Sheet Coordinates 25 : 45 : 428

OWNER IDENTIFICATION - Owner KIN-BIC INC.
 Address 200 CENTINIAL AVE.
 City PISCATAWAY State NJ Zip Code

WELL LOCATION - If not the same as owner please give address. Owner's Well No. 2G
 County MIDDLESEX Municipality EDISON TWP Lot No. 400 Block No. 90
 Address 383 Meadows Road, Edison, NJ

TYPE OF WELL (as per Well Permit Categories) MONITORING Date well completed 2 / 15 / 95
 Regulatory Program Requiring Well CERCLA Case I.D. # NJD049860836
 CONSULTING FIRM/FIELD SUPERVISOR (if applicable) Tele. #

WELL CONSTRUCTION

Total depth drilled 15.6 ft.
 Well finished to 15 ft.

Borehole diameter:
 Top 8 in.
 Bottom 8 in.
 Well was finished: ☒ above grade
☐ flush mounted

Finished above grade, casing
 height (stick up) above land
 surface 4 ft.

Is steel protective casing installed?
☐ Yes ☒ No

Static water level after drilling - ft.
 Water level was measured using -
 Well was developed for N/A hours at N/A gpm
 Method of development N/A

Is permanent pumping equipment installed? ☐ Yes ☒ No

Pump capacity N/A gpm
 Pump type: N/A

Grouting Method HSA

Grout Fluid - Type of Rig B-61

Name of Driller Chad Chism

Health and Safety Plan submitted? ☐ Yes ☒ No

Level of Protection used on site (circle one) None D C (B) A
 License No. 0013753-001375

Name of Drilling Company HARDIN-HUBER, INC.

	Depth to Top (ft.) [From land surface]	Depth to Bottom (ft.)	Diameter (inches)	Type and Material
Inner Casing	4	5	2	Sch 40 PVC
Outer Casing (Not Protective Casing)				
Screen (Note slot size)	5	15	2	Sch 40 PVC .010
Tail Piece				
Gravel Pack	3	15.6	8	#00 Ricci
Annular Seal/Grout	0	3	8	Bentonite slurry
Method of Grouting	tremie			

GEOLOGIC LOG (Copies of other geologic logs and/or geophysical logs should be attached.)

0 - 15.6 red dry stiff clay,
 some silt

I certify that I have drilled the above-referenced well in accordance with all well permit requirements and all applicable rules and regulations.

Driller's Signature Chad Chism Date 2/15/95

COPIES: White - DEP Canary - Driller Pink - Owner Goldenrod - Health Dept.

MONITORING WELL RECORD

Well Permit No. 25 - 46505
Atlas Sheet Coordinates 25 : 45 : 428

OWNER IDENTIFICATION - Owner KIN-BIC INC.
Address 222 CENTINIAL AVE.
City PISCATAWAY State NJ Zip Code _____

WELL LOCATION - If not the same as owner please give address. Owner's Well No. 1G
County MIDDLESEX Municipality EDISON TWP Lot No. 400 Block No. 3C
Address 363 Meadows Road, Edison, NJ

TYPE OF WELL (as per Well Permit Categories) MONITORING Date well completed 2 / 15 / 95
Regulatory Program Requiring Well CERCLA Case I.D. # NJD049882836

CONSULTING FIRM/FIELD SUPERVISOR (if applicable) _____ Tele. # _____

WELL CONSTRUCTION

Total depth drilled 15.6 ft.

Well finished to 15 ft.

Borehole diameter:

Top 8 in.

Bottom 8 in.

Well was finished: ☒ above grade

☐ flush mounted

If finished above grade, casing height (stick up) above land surface _____ ft.

Was steel protective casing installed?

☐ Yes ☒ No

Static water level after drilling _____ ft.

Water level was measured using _____

Well was developed for N/A hours at N/A gpm

Method of development N/A

Was permanent pumping equipment installed? ☐ Yes ☒ No

Pump capacity N/A gpm

Pump type: N/A

Drilling Method HSA

Drilling Fluid _____ Type of Rig B-61

Name of Driller Chad Chism

Health and Safety Plan submitted? ☐ Yes ☒ No

Level of Protection used on site (circle one) None D C B A

J. License No. 0013753-001375

Name of Drilling Company HARDIN-HUBER, INC.

I certify that I have drilled the above-referenced well in accordance with all well permit requirements and all applicable State rules and regulations.

Driller's Signature

Chad Chism

Date 2/15/95

COPIES:

White - DEP

Canary - Driller

Pink - Owner

Goldenrod - Health Dept.

	Depth to Top (ft.) [From land surface]	Depth to Bottom (ft.)	Diameter (inches)	Type and Material
Inner Casing	+4	5	2	Sch 40 PVC
Outer Casing (Not Protective Casing)				
Screen (Note slot size)	5	15	2	Sch 40 PVC .020
Tail Piece				
Gravel Pack	3	15.6	8	#2 Ricci
Annular Seal/Grout	0	5	8	Bentonite slurry
Method of Grouting	tremie			

GEOLOGIC LOG

(Copies of other geologic logs and/or geophysical logs should be attached.)

0 - 15.6

red gray dry stiff clay, some silt